DRAINAGE REPORT

(Phase II)

Santa Rosa Boulevard Roadway Improvements
From East of Eglin Air Force Base to
West of SR 30 (US 98/Miracle Strip Parkway SE)

Prepared For:

Okaloosa County



Prepared by:

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July 19, 2024

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INTRODUCTION

This roadway improvement project along Santa Rosa Boulevard from East of Eglin Air Force Base to West of SR 30 (US 98/Miracle Strip Parkway SE) consists of reconstructing the roadway to improve pedestrian safety, drainage conditions, landscaping, and the overall aesthetic of the corridor.

EXISTING CONDITIONS

The existing typical section of Santa Rosa Blvd consists of a 4-lane divided roadway with a grassed median. The existing stormwater conveyance system consists mostly of roadside swales on each side of the roadway with some closed stormwater systems located near outfall locations. The outfalls are located on the north side of Santa Rosa Blvd, across from the beach access parks. The existing stormwater infrastructure does not provide any formal treatment areas for the stormwater runoff. There are also areas of ponding along the south side of Santa Rosa Blvd during high intensity storm events. Within the Right-of-Way of Santa Rosa Blvd there is approximately 11.63 acres of pervious area, and 18.53 acres of impervious areas.

PROPOSED CONDITIONS

The proposed typical sections of Santa Rosa Blvd consist of:

- 1) Eglin AFB to Park Drive (Sta 100+15.99 to Sta 104+59.15): One travel lane in each direction with concrete ribbon curb and roadside swales, 4' sidewalk on north side
- 2) <u>Park Drive to 1st Beach Park (Sta 104+59.15 to Sta 107+32.94)</u>: One travel lane in each direction with center two-way left turn lane, concrete ribbon curb and roadside swales, 14' Multi-Use Trail on north side
- 3) <u>1st Beach Park to East of Amberjack Drive (Sta 107+32.94 to Sta 193+73.16):</u> Two westbound travel lanes and one eastbound travel lane with center two-way left turn lane/paved median, and Type F curb & gutter, 5' sidewalk on north side, 14' Multi-Use Trail on south side
- 4) East of Amberjack Drive to End of Project (Fire Department) (Sta 193+73.16 to Sta 210+81.97): Match existing configuration with two travel lanes in each direction (undivided) and paved median, proposed 5' sidewalk on north side, 14' Multi-Use Trail on south side

From Station 107+67 to Station 192+33, the roadway will be superelevated 2% to the north to allow the stormwater runoff to be collected in a series of roadside swales on the north side of Santa Rosa Blvd, which will reduce the infrequent flooding that occurs along the WB lanes during intense storm events. This will also minimize impacts to the existing landscaping and signage along the south side of Santa Rosa Blvd. The swales will provide treatment (via retention) for the stormwater runoff due to the in situ sandy soils located along the project corridor the stormwater runoff will be able to percolate back into the groundwater table (see attached Geotechnical Borings for soil conditions). Ditch Bottom Inlets will be strategically located within the swales with their grate elevation set slightly higher than the swale bottoms to provide retention volume. The existing outfall locations will be utilized with proposed closed storm water systems.

The curb and gutter section from Station 192+33 to Station 208+60 will have a proposed closed stormwater system that will collect and convey the runoff the existing outfall located across from the Blue Dolphin Beachwalk. All proposed stormwater pipes will be High Density Poly Ethylene (HDPE) to ensure longevity of the proposed drainage infrastructure. The side drains connecting the roadside swales will be Elliptical Reinforced Concrete Pipes (ERCP) in order to meet required cover and keep the swale elevations above the Seasonal High Water Table (SHWT).

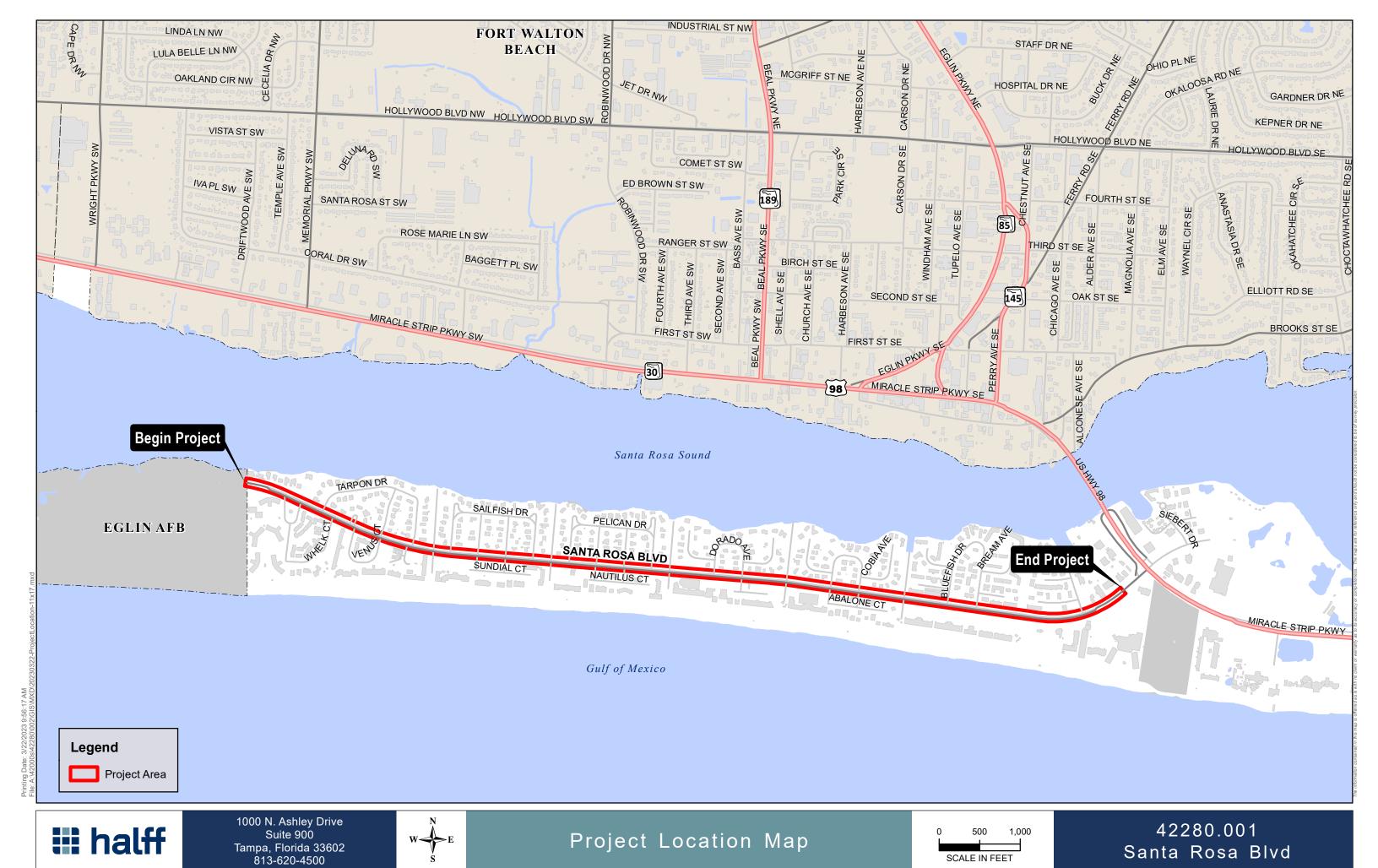
With the proposed improvements along Santa Rosa Blvd, there are approximately 13.97 acres of pervious area and 16.19 acres of impervious areas within the Right-of-Way. Resulting in a net reduction of impervious area of 2.3 acres.

PERMITTING

Halff met with the Northwest Florida Water Management District (NWFWMD) for a Pre-Application meeting on May 22, 2024 to seek guidance on the requirements for an Environmental Resource Permit (ERP) for this project. The NWFWMD determined that due to the size, location, and scope of the project area that this project qualifies for an Individual ERP. Since this project does provide a net reduction in impervious surfaces as well as providing benefits to water quality and water quantity within water resources in the area, the NWFWMD is not requiring the standard stormwater design criteria under an Individual ERP. The calculations and documentation required to meet the permitting requirements will be performed for the next submittal. See attached correspondence with the NWFWMD for required permitting criteria.

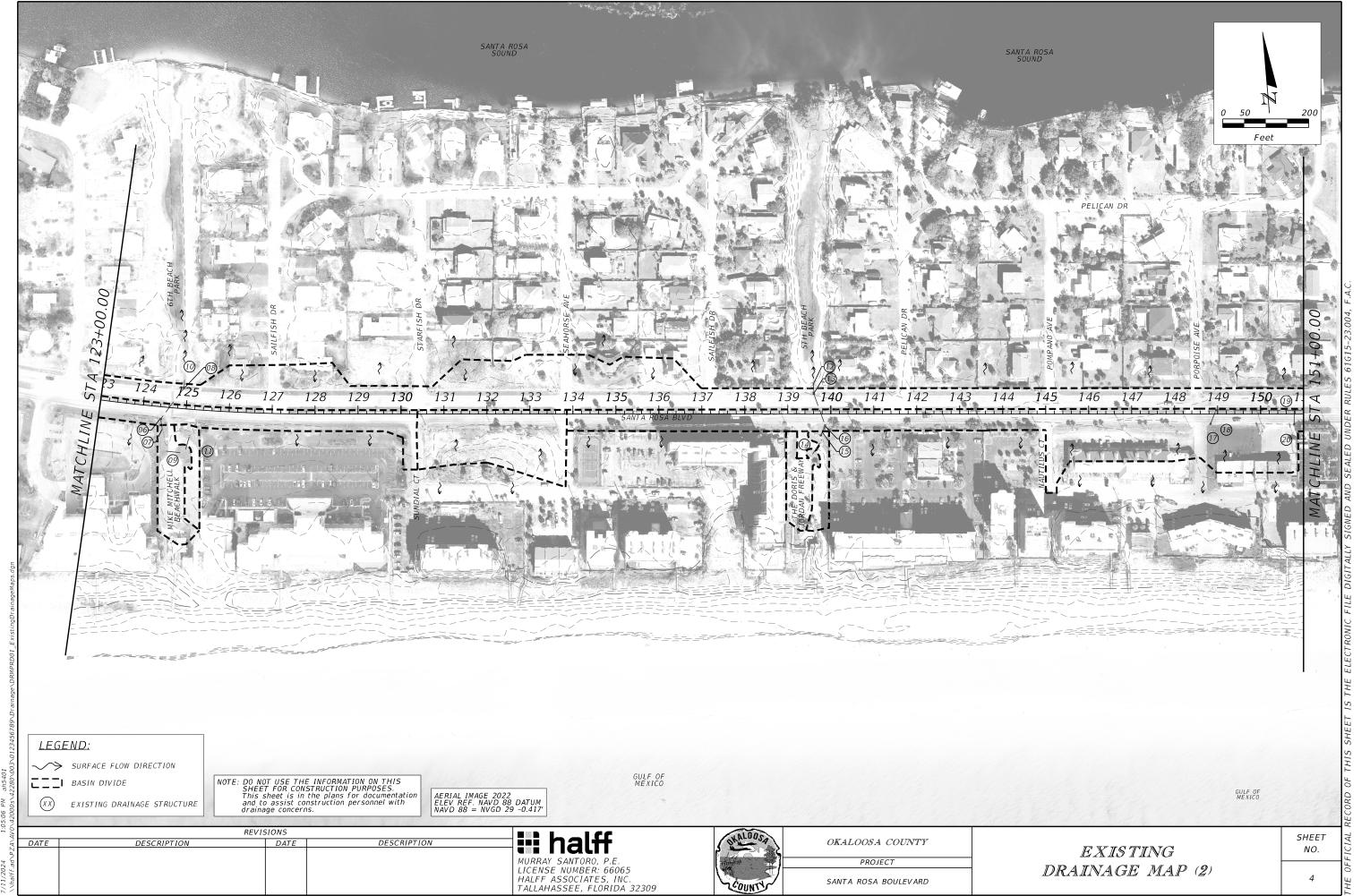
CONCLUSION

The proposed drainage design will utilize roadside swales along the north side of Santa Rosa Blvd to provide treatment for stormwater runoff that was not being formally treated in the existing condition. The roadside swales are hydraulically connected by side drains running through side streets and driveways and have ditch bottom inlets located near the outfall locations. The grate elevations of the ditch bottom inlets are raised above the swale bottom to provide the required treatment volume. A closed drainage system is proposed on the eastern portion of the project where there is curb and gutter.





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TRENCH DRAIN GRATE EL. = 5.20'

DBI GRATE EL. = 4.99' INV. RT. = 2.65' (18" CMP) INV. AH. = 2.68' (15" CMP) INV. BK. = 2.61' (12" CMP)

DBI GRATE EL. = UNK INV. EL. = 2.98' (18" CMP)

DBI GRATE EL. = UNK INV. BK. = 2.52' (15" CMP) INV. LT. = 2.36' (18" CMP) INV. LT. = 2.36' (18" CMP)

FL. EL. = 2.13' (18" CMP) FL. EL. = 2.12' (18" CMP)

DBI GRATE EL. = UNK INV. LT. = 3.74' (15" CMP) INV. RT. = 3.56' (12" CPP)

PIPE INV. EL. = 4.12' (12" CPP)

DBI GRATE EL. = UNK INV. LT. = 3.35' (24" CMP) INV. RT. = 3.18' (15" CMP)

PIPE INV. EL. = 5.35' (12" CPP) (28)

PIPE INV. EL. = 2.95' (24" CMP)

PIPE INV. EL. = 5.53' (12" CPP) (29)

PIPE INV. EL. = 3.10' (24" CMP) 30

GRATE EL. = UNK INV. LT. = 3.13' (24" CMP) INV. RT. = 3.31' (15" CMP)

PIPE INV. EL. = 5.47' (12" CPP)

GRATE EL. = UNK INV. EL. = 4.67' (12" CPP) (33)

DBI GRATE EL. = UNK INV. LT. = 3.76' (15" CMP) INV. RT. = 3.74' (12" CPP)

GRATE EL. = UNK INV. EL. = 4.36' (15" CMP)

DBI GRATE EL. = UNK INV. AH. = 4.18' (24" CMP) INV. RT. = 4.11'(15" CMP)

DBI GRATE EL. = UNK INV. AH. = 3.77' (24" CMP) INV. BK. = 3.99' (24" CMP) INV. RT. = 4.28' (18" CMP)

GRATE EL. = UNK INV. EL. = 4.15' (18" CMP)

NO END TREATMENT

INV. EL. = 12.14' (12" CPP")

HEADWALL INV. EL. = 4.11 (24" CMP) INV. EL. = 3.61' (24" CMP) (41)

DBI INV. LT. = 4.52' (24" CMP) INV. RT. = 4.58' (12" CPP)

INV. EL. = 5.31 (12'' CPP'')

INV. LT. = 5.92' (15" CMP) INV. RT. = 6.06' (12" CPP)

INV. AH. = 5.01' (15" CMP) INV. LT. = 5.15' (12" CPP) INV. RT. = 5.32' (15" CMP)

GRATE EL. = UNK INV EL. = 5.85' (18" CMP) INV EL. = 5.75' (18" CMP)

GRATE EL. = UNK INV. EL. = 4.20' (12" CMP)

TRENCH DRAIN GRATE EL. = UNK INV. EL. = 3.76' (12" CMP) (48)

INV. LT. = 3.56' (12" CMP) INV. BK. = 3.76' (12" CMP) INV. RT. = 3.95' (12" CMP)

TRENCH DRAIN GRATE EL. = UNK INV. EL. = 4.30' (12" CPP) 50

DBI GRATE EL. = UNK INV. EL. = 4.21' (12" CPP)

GRATE EL. = UNK INV. BK. = 4.13' (12" CPP) INV. LT. = 3.57' (12" CPP)

INV. EL. = 0.56' (30" HDPE) (52)

DBI GRATE EL. = UNK INV. EL. = 4.55' (8" PVC)

GRATE EL. = UNK INV. LT. = 1.14' (30" HDPE) INV. RT. = 1.17' (15" CMP)

MANHOLE INV. LT. = 1.01' (15" UNK) INV. RT. = 1.04' (15" CMP)

DBI GRATE EL. = UNK INV. LT. = 1.42' (15" CMP) INV. RT. = 1.67' (12" CMP) INV. AH. = 1.95' (15" CMP)

DBI GRATE EL. = UNK INV. BK. = 3.54' (8" PVC) INV. LT. = 2.55' (12" CMP)

INV. EL. = 4.52' (12'' CPP)

DBI GRATE EL. = UNK INV. BK. = 4.66' (12" CPP) INV. AH. = 4.54' (12" CPP)

DBI GRATE EL. = UNK INV. BK. = 3.99' (12" CPP) INV. AH. = 4.56' (12" CPP)

INV. EL. = 4.56' (12'' CPP)

GRATE EL. = UNK INV. EL. = 3.13' (15" CMP)

DBI GRATE EL. = UNK INV. LT. = 2.43' (15" CMP) INV. RT. = 2.42' (15" CMP) INV. AH. = 2.18' (15" CMP) INV. BK. = 2.07' (15" CMP)

DRAINGRATE GRATE EL. = UNK INV. EL. = 3.43' (15" CMP)

GRATE EL. = UNK INV. AH. = 2.94' (15" CMP) INV. RT. = 2.86' (15" CMP)

DBI GRATE EL. = UNK INV. RT. = 3.05' (15" CMP) INV. AH. = 2.79' (15" CMP) INV. LT. = 2.97' (15" CMP) INV. BK. = 2.87' (15" CMP)

GRATE EL. = UNK INV. EL. = 3.77' (15" CMP)

GRATE EL. = UNK INV. EL. = 3.66' (15" CMP)

MANHOLE INV. BK. = 3.62' (15" CMP) INV. LT. = 3.05' (15" CMP)

TRENCH DRAIN GRATE EL. = UNK

INV. EL. = 4.59' (6'' PVC)

GRATE EL. = UNK INV. RT. = 3.03' (15" CMP) INV. BK. = 3.09' (15" CMP)

DBI GRATE EL. = UNK INV. LT. = 3.16' (15" CMP) INV. AH. = 3.12' (6" PVC) INV. BK. = 3.14' (15" CMP)

DBI GRATE EL. = UNK INV. AH. = 3.23' (15" CMP) INV. BK. = 2.96' (15" CMP)

TRENCH DRAIN GRATE EL. = UNK INV. AH. = 5.63' (8" PVC) INV. BK. = 5.18' (8" PVC)

GRATE EL. = UNK INV. EL. = 3.33' (15" CMP)

DBI GRATE EL. = UNK INV. EL. = 3.70' (15" CMP)

MANHOLE INV. BK. = 3.42' (15" CMP) INV. RT. = 3.61' (15" RCP)

DBI GRATE EL. = UNK INV. EL. = 3.55' (15" RCP)

INV. EL. = 5.17' (8'' PVC)

INV. EL. = 5.71' (8" PVC)

INV. EL. = 5.52' (8" PVC)

INV. EL. = 5.51' (8" PVC)

GRATE EL. = UNK INV. EL. = 3.22' (15" CMP)

GRATE EL. = UNK INV. EL. = 3.26' (15" CMP)

OKALOOSA COUNTY

PROJECT SANTA ROSA BOULEVARD

EXISTING DRAINAGE MAP (5) SHEET NO.

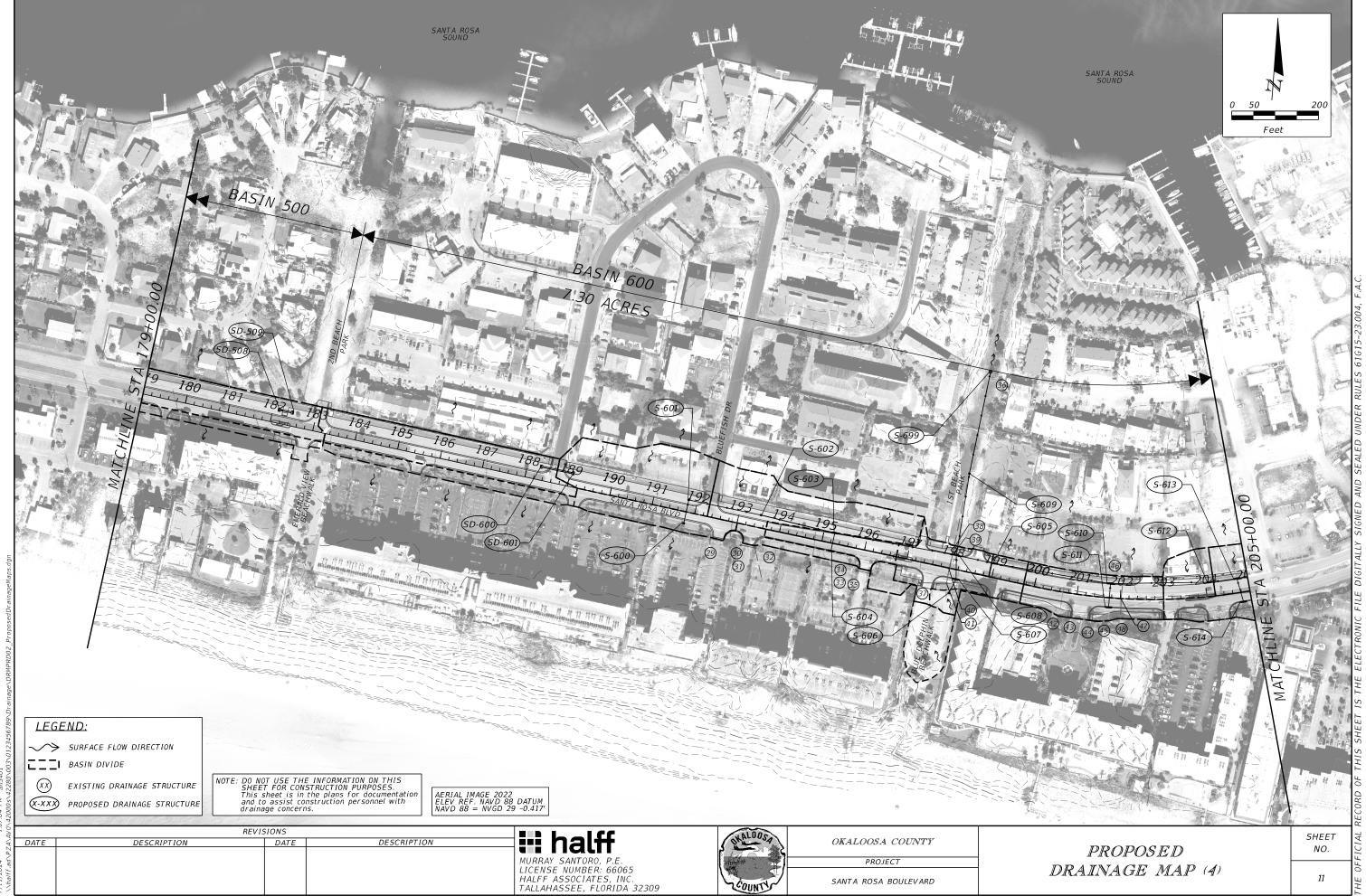


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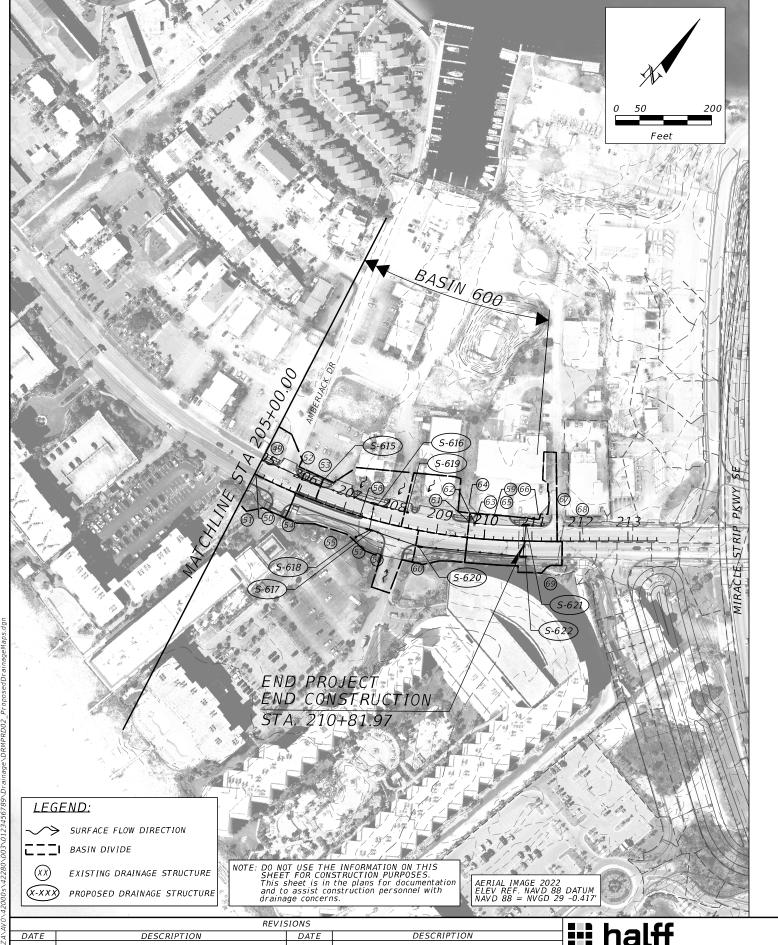


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1005/2 MM NO.70-1



BASIN 100

5-100	0.16 AC
S-101	0.25 AC
S-103	0.17 AC
S-106	0.28 AC
5-107	0.43 AC

BASIN 600

_ ,					
S-600	0.39 AC	S-608	0.26 AC	5-616	0.39 AC
5-601	2.05 AC	5-609	N/A	5-617	0.42 AC
S-602	0.99 AC	5-610	0.39 AC	5-618	N/A
S-603	0.17 AC	5-611	0.34 AC	5-619	0.32 AC
S-604	0.15 AC	5-612	0.28 AC	5-620	0.14 AC
S-605	0.16 AC	5-613	0.40 AC	S-621	0.28 AC
S-606	0.08 AC	5-614	0.40 AC	5-622	0.26 AC
5-607	0.35 AC	S-615	0.05 AC		

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OKALOOSA COUNTY

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SANTA ROSA BOULEVARD

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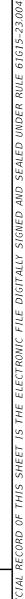
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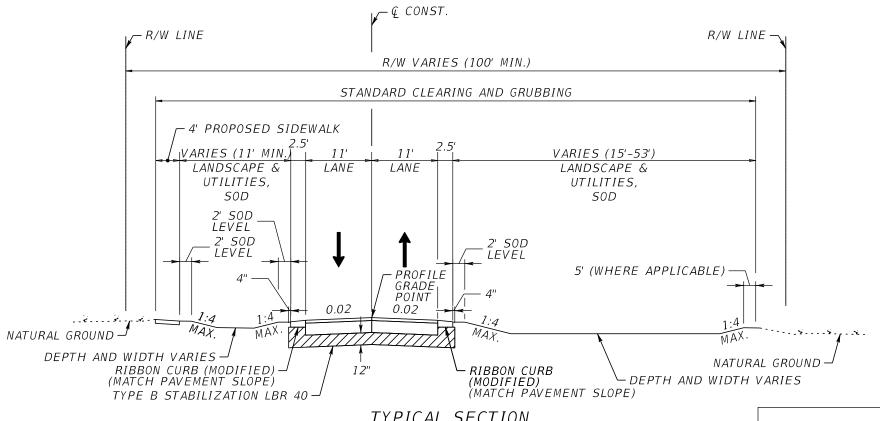
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MURRAY SANTORO, P.E.
LICENSE NUMBER: 66065
HALFF ASSOCIATES, INC.
TALLAHASSEE, FLORIDA 32309

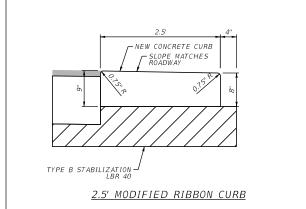
PROPOSED

DRAINAGE MAP (5)





TYPICAL SECTION
SANTA ROSA BLVD
STA. 100+15.99 TO STA. 104+59.15



TRAFFIC DATA

CURRENT YEAR = 2022 AADT = 14200 ESTIMATED OPENING YEAR = N/A AADT = N/A ESTIMATED DESIGN YEAR = N/A AADT = N/A K = 9.0% D = 57.9% T = 2.7% (24 HOUR) DESIGN HOUR T = % TARGET SPEED = 20 MPH DESIGN SPEED = 20 MPH POSTED SPEED = 20 MPH

TRAVEL LANE RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE)

SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")
FRICTION COURSE FC-12.5 (TRAFFIC B) (1.5") (PG 76-22)

COMMERCIAL DRIVEWAY & SIDE STREET RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

RESIDENTIAL DRIVEWAY RECONSTRUCTION

6" CONCRETE

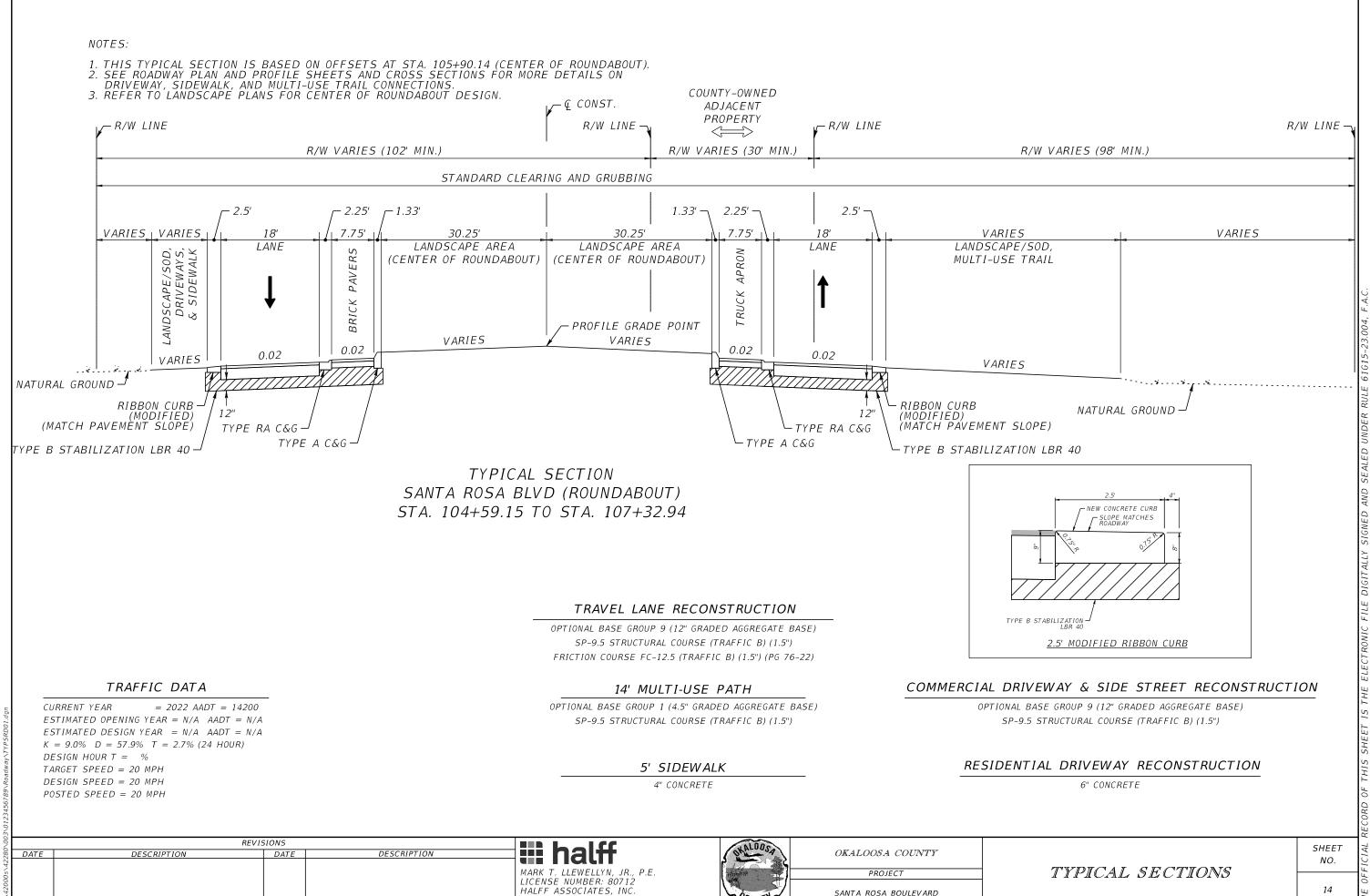
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				MARK T. LLEWELLYN, JR., P.E. LICENSE NUMBER: 80712 HALFF ASSOCIATES, INC. TALLAHASSEE, FLORIDA 32309



OKALOOSA COUNTY	
PROJECT	
SANTA ROSA BOULEVARD	

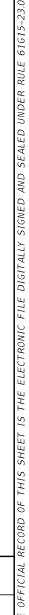
TYPICAL SECTIONS

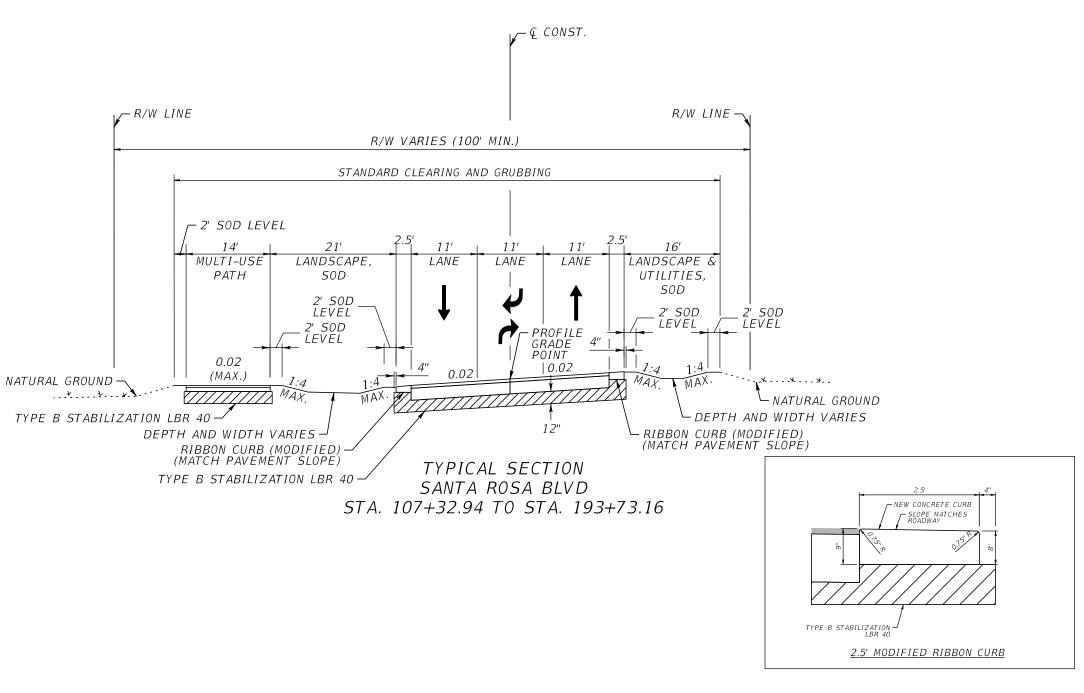
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HALFF ASSOCIATES, INC. TALLAHASSEE, FLORIDA 32309







TRAFFIC DATA

CURRENT YEAR = 2022 AADT = 14200 ESTIMATED OPENING YEAR = N/A AADT = N/A ESTIMATED DESIGN YEAR = N/A AADT = N/A K = 9.0% D = 57.9% T = 2.7% (24 HOUR) DESIGN HOUR T = % TARGET SPEED = 35 MPH DESIGN SPEED = 35 MPH POSTED SPEED = 35 MPH

TRAVEL LANE RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE)

SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

FRICTION COURSE FC-12.5 (TRAFFIC B) (1.5") (PG 76-22)

14' MULTI-USE PATH

OPTIONAL BASE GROUP 1 (4.5" GRADED AGGREGATE BASE)
SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

COMMERCIAL DRIVEWAY & SIDE STREET RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

RESIDENTIAL DRIVEWAY RECONSTRUCTION

6" CONCRETE

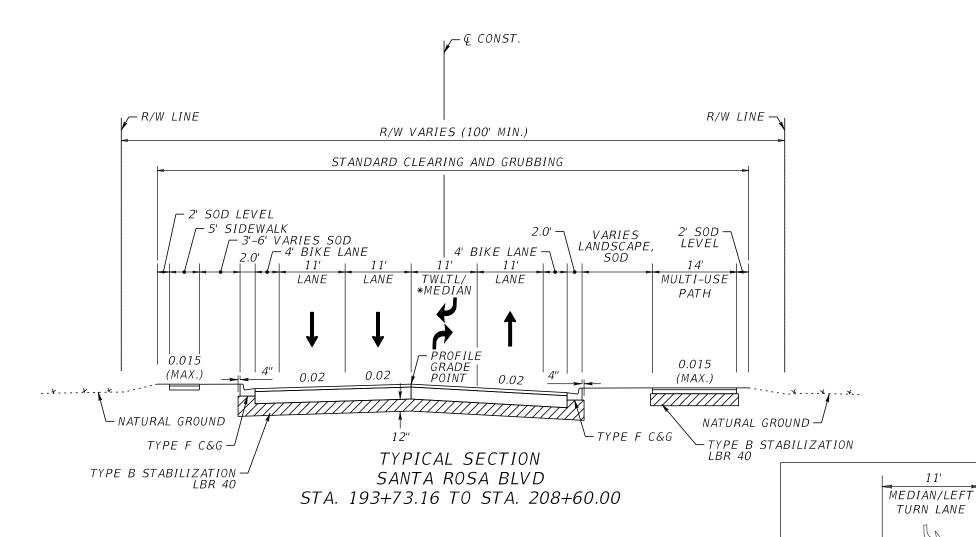
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				MARK T. LLEWELLYN, JR., P.E. LICENSE NUMBER: 80712
				HALFF ASSOCIATES, INC.
				TALLAHASSEE, FLORIDA 32309



OKALOOSA COUNTY	
PROJECT	
SANTA ROSA ROULEVARD	

SHEET NO.





TRAVEL LANE RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5") FRICTION COURSE FC-12.5 (TRAFFIC B) (1.5") (PG 76-22)

14' MULTI-USE PATH

OPTIONAL BASE GROUP 1 (4.5" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

4" CONCRETE

5' SIDEWALK

COMMERCIAL DRIVEWAY & SIDE STREET RECONSTRUCTION

TYPE E C&G

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

*MEDIAN

STA. 201+48.70 TO STA. 207+43.13 N.T.S.

11'

-TYPE E C&G

-SLOPE VARIES

RESIDENTIAL DRIVEWAY RECONSTRUCTION

6" CONCRETE

TRAFFIC DATA

CURRENT YEAR = 2022 AADT = 14200ESTIMATED OPENING YEAR = N/A AADT = N/AESTIMATED DESIGN YEAR = N/A AADT = N/AK = 9.0% D = 57.9% T = 2.7% (24 HOUR) $DESIGN\ HOUR\ T = \%$ TARGET SPEED = 35 MPHDESIGN SPEED = 35 MPH POSTED SPEED = 35 MPH

DESCRIPTION

DATE

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			MARK T. LLEWELLYN, JR., P.E.
			LICENSE NUMBER: 80712
			HALFF ASSOCIATES, INC.
			TALLAHASSEE, FLORIDA 3230.



OKALOOSA COUNTY
PROJECT
SANTA ROSA ROUJEVARD

TYPICAL SECTIONS

SHEET NO.

TRAVEL LANE RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE)

SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

FRICTION COURSE FC-12.5 (TRAFFIC B) (1.5") (PG 76-22)

14' MULTI-USE PATH

OPTIONAL BASE GROUP 1 (4.5" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

5' SIDEWALK 4" CONCRETE

COMMERCIAL DRIVEWAY & SIDE STREET RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

RESIDENTIAL DRIVEWAY RECONSTRUCTION

6" CONCRETE

TRAFFIC DATA

CURRENT YEAR = 2022 AADT = 14200ESTIMATED OPENING YEAR = N/A AADT = N/AESTIMATED DESIGN YEAR = N/A AADT = N/AK = 9.0% D = 57.9% T = 2.7% (24 HOUR) DESIGN HOUR T = %TARGET SPEED = 35 MPH DESIGN SPEED = 35 MPH

POSTED SPEED = 35 MPH

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DATE	DESCRIPTION	DATE	DESCRIPTION	iii nalii
				MARK T. LLEWELLYN, JR., P.E.
				LICENSE NUMBER: 80712
				HALFF ASSOCIATES, INC.
				TALLAHASSEE, FLORIDA 32309

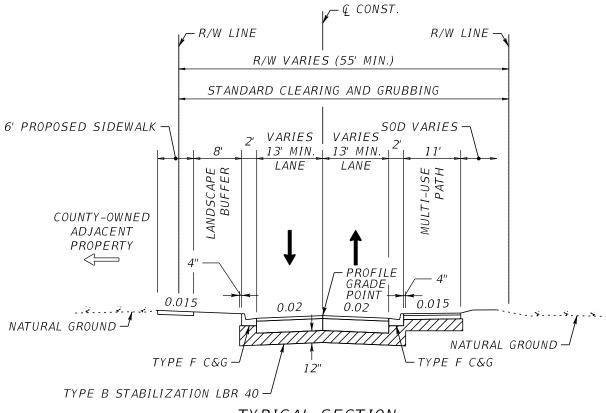


OKALOOSA COUNTY
PROJECT
SANTA ROSA BOULEVARD

SHEET	
NO.	

17

7/19/2024 5:43:12 PM ah3923 AN 420000-X 433800 0030 01234567800 004 1. 6' PROPOSED SIDEWALK CONTINUES TO STA. 35+92.06.



TYPICAL SECTION PARK DRIVE STA. 30+00.00 TO STA. 32+75.43

TRAVEL LANE RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5") FRICTION COURSE FC-12.5 (TRAFFIC B) (1.5") (PG 76-22)

14' MULTI-USE PATH

OPTIONAL BASE GROUP 1 (4.5" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

5' SIDEWALK 4" CONCRETE

COMMERCIAL DRIVEWAY & SIDE STREET RECONSTRUCTION

OPTIONAL BASE GROUP 9 (12" GRADED AGGREGATE BASE) SP-9.5 STRUCTURAL COURSE (TRAFFIC B) (1.5")

RESIDENTIAL DRIVEWAY RECONSTRUCTION

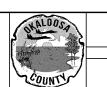
6" CONCRETE

TRAFFIC DATA

CURRENT YEAR = 2022 AADT = 14200ESTIMATED OPENING YEAR = N/A AADT = N/AESTIMATED DESIGN YEAR = N/A AADT = N/AK = 9.0% D = 57.9% T = 2.7% (24 HOUR) DESIGN HOUR T = %TARGET SPEED = 20 MPH DESIGN SPEED = 20 MPH

POSTED SPEED = 20 MPH

DATE	DESCRIPTION	DATE	DESCRIPTION	— ::: nal n
				MARK T. LLEWELLYN, JR., P.E. LICENSE NUMBER: 80712 HALFF ASSOCIATES, INC. TALLAHASSEE, FLORIDA 32309



OKALOOSA COUNTY	
PROJECT	

SANTA ROSA BOULEVARD

SHEET	
NO.	



JOB:	Santa Rosa Blvd	COUNTY:	Okaloosa	SHEET
FPID:		DISTRICT		1
CALC'D BY:	MJA	DATE:	31-May-24	OF
CHECKID BY:	MADC	DATE	2 lun 24	4

INLET SPREAD CALCULATIONS

Reference: Section 6.3.2.4, Chapter 6, FDOT Drainage Design Guide, 2023

			SLO	PES		Reference: Section 6.3.2.4, Chapter 6, FDOT Drainage Design Guide, 2023 FLOWS											SPREAD				
STRUCTURE	INLET	STATION	LONG.	CROSS		ROA	ADWAY		GUTTER MAX	BYPASS-IN	TOTAL IN	INTERCEPT	BYPA	SS-OUT							
NUMBER	TYPE	(ft.)	5	S _x	RUNOFF COEFFICIENT	INTENSITY	AREA	Q _r	Q_g	Qbi	Qt	Qı	Q _{bo}	To Structure	ALLOWABLE SPREAD (T)	GUTTER SPREAD (T _{gutter})	GUTTER SPREAD CHECK	COMMENTS			
l			<u> </u>						Proposed Co	onditions											
5-622	Type 6	210+82 RT	0.30%	2.00%	0.95	4.00 in./hr	0.26 ac.	0.99 cfs	1.69 cfs	0.00 cfs	0.99 cfs	0.83 cfs	5	AG	11.00 ft.	8.99 ft.	OK				
I		I			I									I				l			
5-621	Туре 6	210+87 LT	0.30%	2.00%	0.95	4.00 In./hr	0.28 ac.	1.06 cfs	1.69 cfs	0.00 cfs	1.06 cfs	0.88 cfs	5	:AG	11.00 ft.	9.25 ft.	OK				
									<u>'</u>		1	<u>'</u>		•							
S-620	Type 5	208+68 RT	0.30%	2.00%	0.95	4.00 in./hr	0.14 ac.	0.53 cfs	1.69 cfs	0.00 cfs	0.53 cfs	0.49 cfs	0.04 cfs	S-617	11.00 ft.	7.13 ft.	OK				
S-617	Type 6	207+65 RT	0.30%	2.00%	0.95	4.00 in./hr	0.42 ac.	1.60 cfs	1.69 cfs	0.04 cfs	1.64 cfs	1.25 cfs	9	AG	11.00 ft.	10.87 ft.	OK				
S-619	S-Gutter	208+64 LT	0.30%	2.00%	0.95	4.00 in./hr	0.32 ac.	1.22 cfs	1.69 cfs	0.00 cfs	1.22 cfs	1.06 cfs	0.15 cfs	S-616	11.00 ft.	9.72 ft.	OK				
S-616	Type 6	207+65 LT	0.30%	2.00%	0.95	4.00 in./hr	0.38 ac.	1.44 cfs	1.69 cfs	0.15 cfs	1.60 cfs	1.23 cfs	5	AG	11.00 ft.	10.77 ft.	OK				
<i>S-615</i>	Type 5	206+50 LT	0.30%	2.00%	0.95	4.00 In./hr	0.05 ac.	0.19 cfs	1.69 cfs	0.00 cfs	0.19 cfs	0.19 cfs	0.00 cfs	S-616	11.00 ft.	4.85 ft.	OK				
		1	1	T.	1	T	T	T			Т	T						1			
5-612	Type 5	204+12 LT	0.30%	2.00%	0.95	4.00 in./hr	0.27 ac.	1.03 cfs	1.69 cfs	0.00 cfs	1.03 cfs	0.86 cfs	0.17 cfs	5-613	11.00 ft.	9.12 ft.	OK				
5-613	Туре 6	204+75 LT	0.30%	2.00%	0.95	4.00 in./hr	0.39 ac.	1.48 cfs	1.69 cfs	0.17 cfs	1.65 cfs	1.26 cfs		AG	11.00 ft.	10.90 ft.	OK				
		T	T											1				ı			
5-614	Type 6	204+75 RT	0.30%	2.00%	0.95	4.00 in./hr	0.39 ac.	1.48 cfs	1.69 cfs	0.00 cfs	1.48 cfs	1.16 cfs	-	AG	11.00 ft.	10.47 ft.	OK				
S-610	Туре 6	201+75 LT	0.30%	2.00%	0.95	4.00 in./hr	0.38 ac.	1.44 cfs	1.69 cfs	0.00 cfs	1.44 cfs	1.13 cfs		:AG	11.00 ft.	10.37 ft.	ок				
3-010	Type 0	201473 EI	0.50%	2.00%	0.55	4.00 111./111	0.56 ac.	1.44 (13	1.03 (73	0.00 073	1.44 (/3	1.13 (15	•	Au	11.00 11.	10.57 11.	UK .				
5-611	Туре 6	201+75 RT	0.30%	2.00%	0.95	4.00 In./hr	0.38 ac.	1.44 cfs	1.69 cfs	0.00 cfs	1.44 cfs	1.13 cfs		:AG	11.00 ft.	10.37 ft.	OK				
S-605	Type 5	199+04 LT	0.30%	2.00%	0.95	4.00 in./hr	0.10 ac.	0.38 cfs	1.69 cfs	0.00 cfs	0.38 cfs	0.36 cfs	0.02 cfs	5-608	11.00 ft.	6.28 ft.	OK				
5-608	Туре 6	198+10 LT	0.30%	2.00%	0.95	4.00 In./hr	0.26 ac.	0.99 cfs	1.69 cfs	0.02 cfs	1.01 cfs	0.84 cfs	9	AG	11.00 ft.	9.05 ft.	OK				
S-607	Type 6	198+20 RT	0.30%	2.00%	0.95	4.00 in./hr	0.44 ac.	1.67 cfs	1.69 cfs	0.00 cfs	1.67 cfs	1.27 cfs		:AG	11.00 ft.	10.95 ft.	OK				
<u> </u>									<u> </u>		·	•				<u> </u>					
<i>S-603</i>	Type 6	195+25 LT	0.30%	2.00%	0.95	4.00 in./hr	0.17 ac.	0.65 cfs	1.69 cfs	0.00 cfs	0.65 cfs	0.58 cfs	0.07 cfs		11.00 ft.	7.67 ft.	OK				
5-604	Type 6	195+25 RT	0.20%	2.00%	0.95	4.00 In./hr	0.15 ac.	0.57 cfs	1.38 cfs	0.07 cfs	0.64 cfs	0.56 cfs	0.07 cfs		11.00 ft.	8.23 ft.	OK				

FLORIDA DEPARTMENT OF TRANSPORTATION STORM DRAIN TABULATION FORM

Financial Prj Id. County: Okaloosa County Network: BSN 600 Designed by MJA Description: Basin 600 Organization HALFF ASSOCIATES State Road: Santa Rosa Blvd Checked by: MBS Date: 6/28/2024

LOCATION	STR.	TYPE	LEN.	AREA	S (Ac)	SUB-	TIME	TIME	INTEN	TOTAL	BASE	TOTAL	MINOR	INLET	HGL	HYDRA	AULIC GR	ADE	# PIPI	SLOPE	ACTUAI	FULL	NOTES & REMARKS
OF	NO.	OF		C=		TOTAL		OF					LOSS				CROWN		3 SIZI		_	FLOW	ZONE: 1
UPPER END		STR.		C=	0.50	(C*A)	CONC	FLOW		(- /	SUMM						OWLINE		R (in.)			CAP.	FREQ. (Yrs): 10
ALIGNMENT NAME	UPPER			C=	0.95	`		SECT.			BASE					UPPER	LOWER				VEL.		MANNINGS n: 0.0120
STATION DIST SE	LOWER		(ft.)	INC	TOTAL		(min)	(min)	(in/hr)		(cfs)	(cfs)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)			N MIN.	(fps)	(cfs)	TAILW EL (ft): 3.00
-L-	S-600			0.22	0.22	0.04		` '								5.57	5.56	0.01	18.0	0.025	0.81		Located in Swale. Check I
		DBI-C	53.10	0.00	0.00	0.00	10.00	0.44	6.97	0.21	0.00	1.43	0.01	6.00	0.43	3.10	2.90		1	0.377		6.98	ocation of the exist WM
191+80 27.00 Rt	S-601			0.17	0.17	0.16					0.00					1.60	1.40	0.20	18.0		_		
-L-	S-601			0.00	0.22	0.00					0.00					5.56				0.096			Base flow added from ICPR
	-	DBLC	167.00		0.00	0.00	10.44	1.39	6.87	0.21	1.67	3.09	0.04	5.00	-0.56	2.90	2.65		1	0.150	-		. Located in Swale
191+80 28.10 Lt.	S-602	DDI-O	107.00	0.00	0.17	0.00	10.44	1.00	0.07	0.21	1.67	0.00	0.04	0.00	0.00	1.40		0.25	18.0			4.40	. Located in Owaic
-L-	S-602			0.18	0.40	0.04					1.07					5.40	4.70	0.69	18.0				Located in Swale
_	0 002	DBI-C	172.00		0.58	0.29	11.83	0.77	6.59	0.75	0.00	6.62	0.11	5.00	-0.40	2.65	2.40	0.00	1 10.0	0.145	-	4.34	- Swale
193+50 29.00 Lt.	S-603	DBI-C	172.00	0.33	0.30	0.23	11.03	0.77	0.59	0.73	1.67	0.02	0.11	3.00	-0.40	1.15	0.90	0.25	18.0			4.54	ı
-L-	S-603			0.23	0.40	0.22					1.07					4.70	4.29	0.23	24.0				
-L-	3-003	D.C.	202.00				40.00	4 74	C 45	4.05	0.00	0.47	0.00	0.05	4 55		-	0.42	24.0			40.00	ı
105,05,00	0 600	P-6	282.00		0.58	0.00	12.60	1.74	6.45	1.05	0.00	8.47	0.08	6.25	1.55	2.90	2.40	0.50	1 24 2	0.177		10.32	₁
195+25.00 22.78 Lt.				0.17	0.72	0.16					1.67					0.90	0.40	0.50	24.0				
-L-	S-604			0.00	0.00	0.00										4.71	4.70	0.01	18.0		=		ı
		P-6	39.00		0.00	0.00	10.00	0.32	6.97	0.14	0.00	0.99	0.00	6.37	1.66	2.60	2.40		1	0.513	_	8.15	ı
195+25 16.50 Rt				0.15	0.15	0.14					0.00					1.10		0.20	18.0	_			
-L-	S-605			0.04	0.47	0.01										4.46	4.29	0.17	36.0	0.188	3.22		ı
		P-5	91.00	0.00	0.23	0.00	18.14	0.47	5.64	3.69	0.00	22.77	0.08	6.32	1.86	3.60	3.40		1	0.220		33.87	ı
199+04.00 31.50 Lt.	S-608			0.06	3.66	0.06					2.00					0.60	0.40	0.20	36.0	0.059	4.79		
-L-	S-606			0.08	0.08	0.02										4.32	4.32	0.00	18.0	0.003	0.25		
	_	DBI-C	33.00	0.00	0.00	0.00	10.00	0.28	6.97	0.06	0.00	0.44	0.00	5.45	1.13	2.90	2.80		1	0.303		6.26	ı
197+98.63 47.88 Rt.	S-607			0.05	0.05	0.05					0.00					1.40	1.30	0.10	18.0	0.150	3.54		ı
-L-	S-607			0.03	0.11	0.01										4.32	4.29	0.03	18.0	0.063	1.27		
		P-6	52.00	0.00	0.00	0.00	10.27	0.43	6.91	0.33	0.00	2.25	0.01	6.30	1.98	2.80	2.70		1	0.192		4.99	ı
198+20.00 20.50 Rt.	S-608			0.27	0.32	0.26					0.00					1.30	1.20	0.10	18.0		_		ı
-L-	S-608			0.04	1.02	0.01										4.29	3.95	0.34	42.0	0.181	3.45		
		J-6	187.00	0.00	0.81	0.00	18.61	0.90	5.58	5.28	0.00	33.15	0.17	6 11	1.82	3.90	3.80		1	0.053	-	25.20	ı
198+09.52 31.50 Lt.	S-609		107.00	0.22	4.92	0.21	10.01	0.00	0.00	0.20	3.67	00.10	0.11	0.11		0.40	0.30	0.10	42.0		_	20.20	ı
-L-	S-609			0.00	1.02	0.00					0.07					3.95	3.65	0.30	42.0				
	0 000	MH_7 I	235.00		0.81	0.00	19.51	0.00	5.48	5.28	0.00	32.61	0.09	4.42	0.47	3.80	3.65	0.00	1	0.064	-	27.54	ı
197+99 223.00 Lt.	S-600	IVII I-7 J	200.00	0.00	4.92	0.00	13.51	0.00	J. 4 U	3.20	3.67	32.01	0.03	7.72	0.47	0.30	0.15	0.15	1/20	0.048	_	21.54	ı
-L-	S-610			0.09	0.43	0.00					3.07					4.81	4.46	0.15		0.040			
	3-0.0	P-6	263.00		0.43	0.02	16.79	1.35	5.81	3.62	0.00	23.02	0.08	5.95	1.14	3.80	3.60	3.00	1 50.0	0.076		19.93	₁
201+75.00 31.50 Lt.	S 605	F-0	203.00	0.00	3.60	0.00	10.79	1.33	5.61	3.02	2.00	23.02	0.00	5.95	1.14	0.80	0.60	0.20	36.0		_	19.93	₁
-L-	S-605 S-611			0.30	0.03	0.29					2.00					4.84	4.81	0.20		0.055			
-L-	3-011	D.C	E4 00				10.00	0.40	6.07	0.20	0.00	2.40	0.04	6 17	1 22	_	_	0.03	10.0		_		₁
204 . 75 00 00 50 50	0.040	P-6	51.00		0.00	0.00	10.00	0.43	6.97	0.30	0.00	2.10	0.01	6.17	1.33	3.30	3.20	0.40	1 40 0	0.196	_	5.04	₁
201+75.00 20.50 Rt.				0.31	0.31	0.29					0.00					1.80	1.70	0.10	18.0				
-L-	S-612		000.55	0.04	0.31	0.01	45.45		- 06		0.00		0.00	- 06	0.00	5.05	4.81	0.24	30.0	0.105	-		₁
		P-5	228.00		0.23	0.00	15.45	1.34	5.99	3.02	0.00	20.08	0.06	5.98	0.93	4.00	3.80		1	0.088	_	21.40	₁
204+12 31.50 Lt.				0.24	2.99	0.23					2.00					1.00	0.80	0.20	36.0				
-L-	S-613			0.04	0.27	0.01										5.14	5.05	0.09	36.0	0.163			₁
		P-6	58.00		0.23	0.00	15.09	0.36	6.04	2.78	0.00	18.81	0.06	5.80	0.66	4.10	4.00		1	0.172	_	30.00	₁
204+75.00 31.50 Lt.	S-612			0.36	2.75	0.34					2.00					1.10	1.00	0.10	36.0	0.059	4.24		

Units: ENGLISH HGL method: Standard FDOT (Jump HGL to pipe crown).

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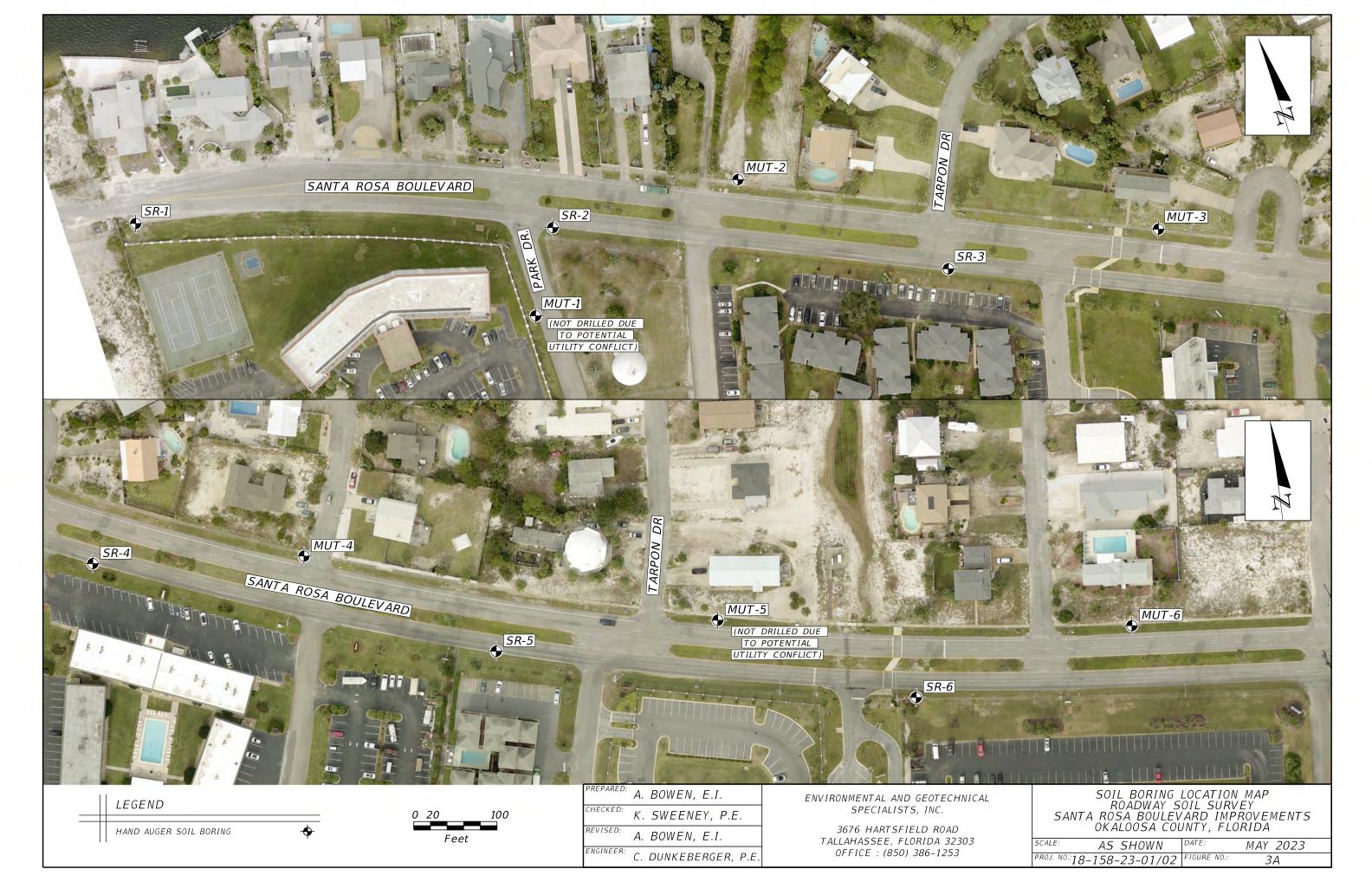
FLORIDA DEPARTMENT OF TRANSPORTATION STORM DRAIN TABULATION FORM

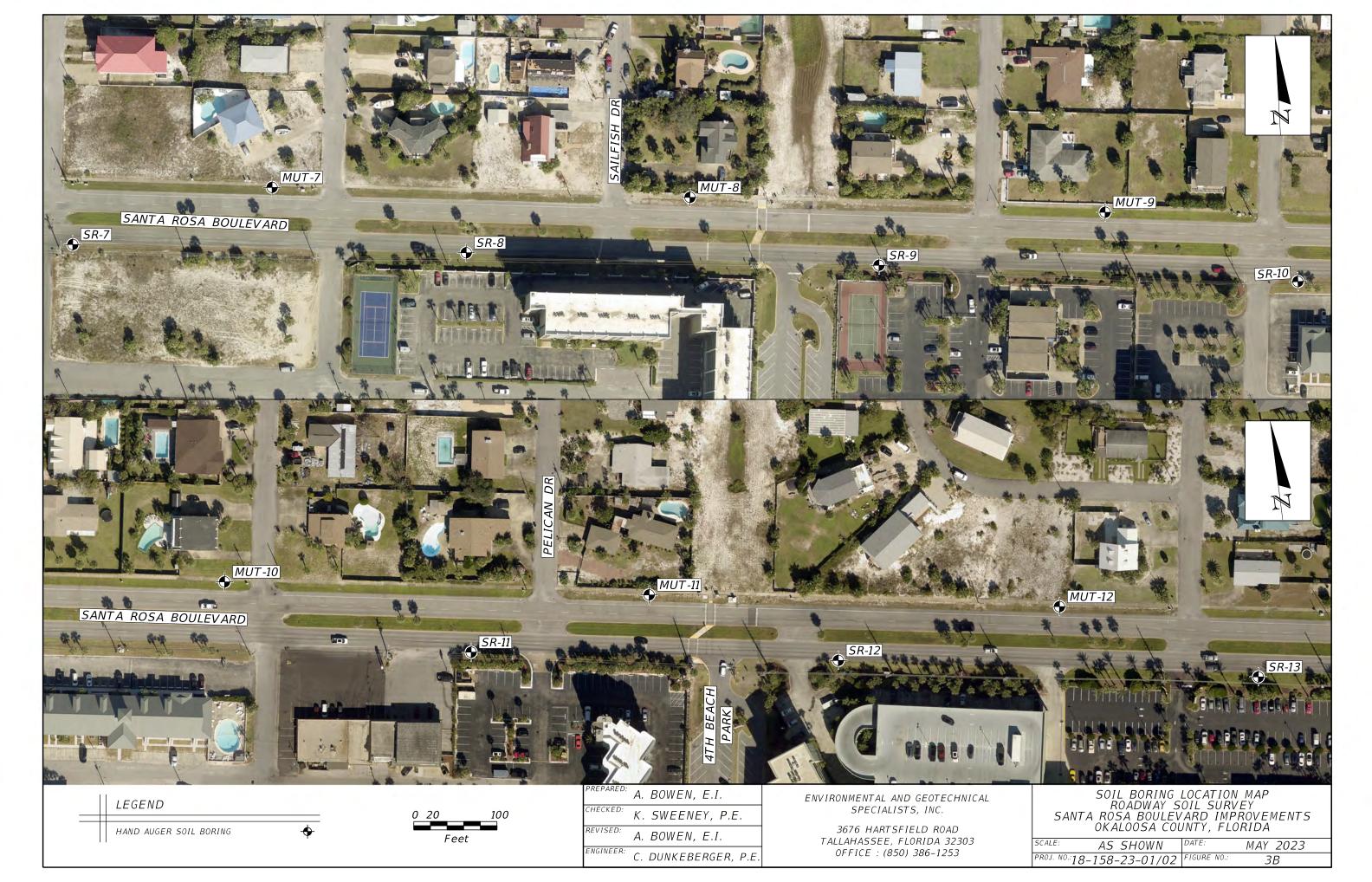
Financial Prj Id . County: Okaloosa County Network: BSN 600 Designed by MJA Description: Basin 600 Organization HALFF ASSOCIATES State Road: Santa Rosa Blvd Checked by: MBS Date: 6/28/2024

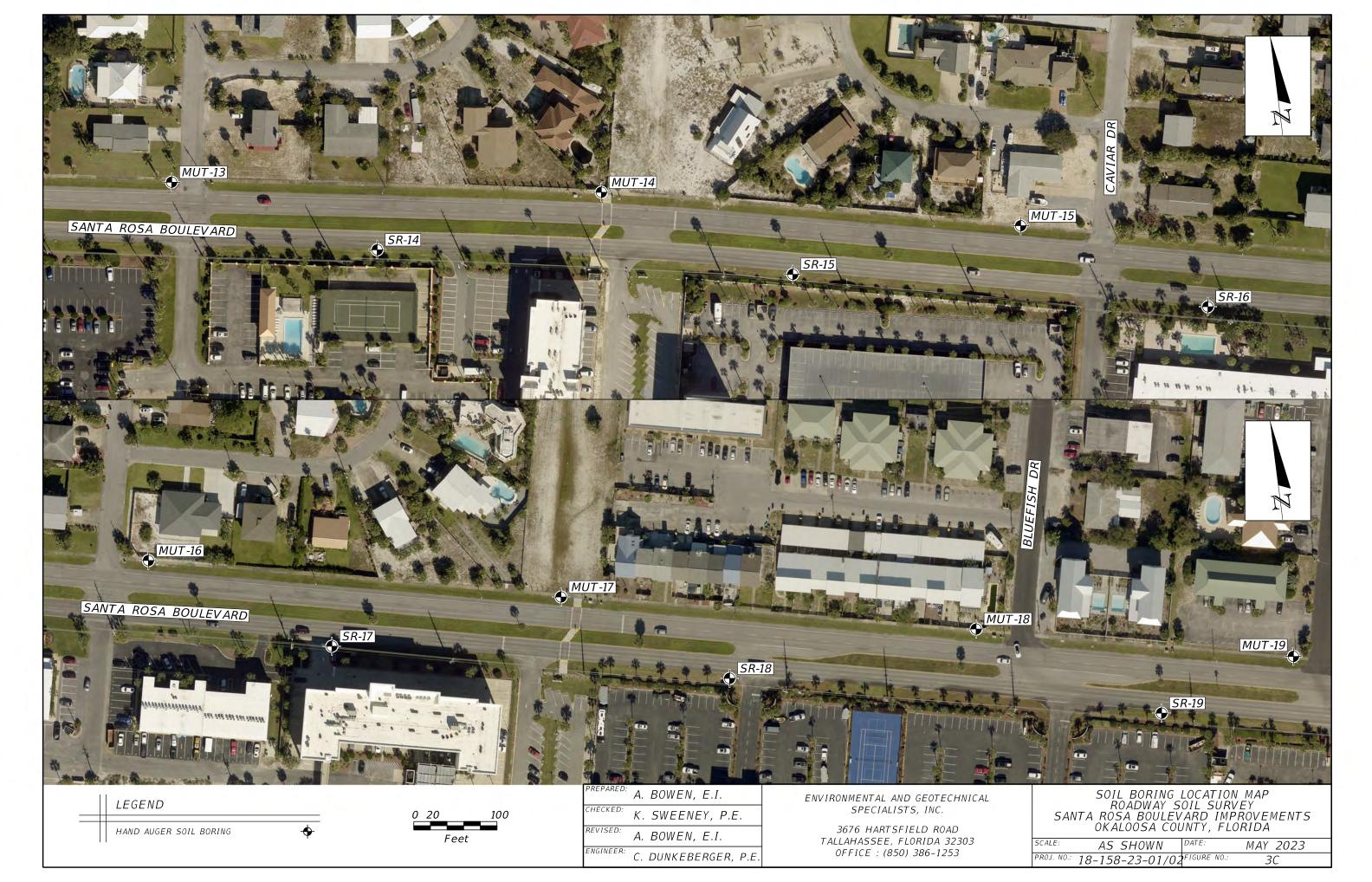
LOCA		STR.	TYPE	LEN.	AREA		SUB-	TIME		INTEN				MINOR				AULIC GR	ADE			OPEAC			NOTES & REM	MARKS
OF	=	NO.	OF		C=		TOTAL	OF	OF		(C*A)	FLOW	FLOW	LOSS	ELEV.	CLEAR		CROWN		BS	ZE			FLOW	ZONE:	1
UPPER	REND		STR.		C=	0.50	(C*A)	CONC	FLOW			SUMM					FL	OWLINE.		R (n.) F	HGL PI	HYS.	CAP.	FREQ. (Yrs):	10
ALIGNMEN	NT NAME	UPPER			C=	0.95			SECT.			BASE					UPPER	LOWER	FALL	LR	SE P	'HYS \	/EL.		MANNINGS n:	0.0120
STATION	DIST S	DLOWER		(ft.)	INC	TOTAL		(min)	(min)	(in/hr)		(cfs)	(cfs)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)	(ft.)	SS	AN N	MIN. (fps)	(cfs)	TAILW EL (ft):	3.00
-L·	-	S-614			0.08	0.08	0.02										5.17	5.14	0.03	18	.00 0	0.055	1.19			
			P-6	52.00	0.00	0.00	0.00	10.00	0.43	6.97	0.30	0.00	2.10	0.01	6.02	0.85	2.70	2.60		1		0.192		4.99		
204+75.00	22.00 F	Rt. S-613			0.30	0.30	0.29					0.00					1.20	1.10	0.10	18	.00 0	0.150	2.82			
-L-	-	S-615			0.01	0.15	0.00										5.39	5.14	0.25	30	.00 0	0.165	3.08			_
		-	P-5	150.00	0.00	0.23	0.00	14.28	0.81	6.17	2.13	0.00	15.14	0.07	5.99	0.60	3.80	3.60		1		0.133		16.23		
206+50.00	33.00 L	t. S-613			0.04	2.09	0.04					2.00					1.30	1.10	0.20	30	.00	0.076	3.31			
-L-	-	S-616			0.02	0.14	0.00										5.60	5.39	0.21	30	.00 0	0.177	3.08			
			P-6	121.00	0.17	0.23	0.09	13.63	0.66	6.27	2.09	0.00	15.11	0.07	5.66	0.06	3.90	3.80		1		0.083		12.77		
207+65	31.50 L	t. S-615			0.20	2.05	0.19					2.00					1.40	1.30	0.10	30	.00	0.076	2.60			
-L·	-	S-617			0.04	0.12	0.01										5.65	5.60	0.05	30	.00 0	0.093	1.85			
		-	P-6	52.00	0.06	0.06	0.03	13.19	0.43	6.34	1.12	0.00	9.09	0.03	5.87	0.22	4.00	3.90		1		0.192		19.49		
207+65	20.50 F	Rt. S-616			0.27	1.12	0.26					2.00				-	1.50	1.40	0.10	30			3.97			
-L·		S-618			0.00	0.00	0.00										5.79	5.65	0.14	12	.00 0		2.55		Offsite (Verify Inv	/ert) A
			MH-7	32.00	0.00	0.00	0.00	10.00	0.21	6.97	0.00	2.00	2.00	0.05	6.40	0.61	2.50	2.40		1		0.313	-	2.16	dd Baseflow	
207+40	44.00 F	Rt. S-617			0.00	0.00	0.00					2.00					1.50	1.40	0.10	12	.00	0.257	2.75			
-L·		S-619			0.00	0.00	0.00										5.79	5.60	0.18				2.74			-
			DBI-C	68.00	0.00	0.00	0.00	10.00	0.41	6.97	0.69	0.00	4.84	0.06	5.98	0.19	3.00	2.90		1		0.147		4.36		
208+32	32.00 L	.t. S-616			0.73	0.73	0.69					0.00					1.50	1.40	0.10	18	.00 0	0.150	2.47			
-L-	-	S-620			0.01	0.08	0.00										5.68	5.65	0.02	30	.00 0	0.023	1.09		Check adj projec	t for any
		-	P-5	106.00	0.00	0.00	0.00	12.31	0.88	6.50	0.82	0.00	5.35	0.01	5.96	0.28	4.00	3.90		1		0.094	-	13.65	flow coiming to u	s
208+68.00	31.50 F	Rt. S-617			0.38	0.85	0.36					0.00					1.50	1.40	0.10	30	.00	0.076	2.78		· ·	
-L-	-	S-621			0.01	0.01	0.00										5.73	5.72	0.01	24	.00 0	0.010	0.57			
			P-6	62.00	0.00	0.00	0.00	10.00	0.52	6.97	0.26	0.00	1.80	0.00	5.91	0.18	3.90	3.75		1		0.242		12.05		
210+86.66	31.50 L	t. S-622			0.27	0.27	0.26					0.00					1.90	1.75	0.15	24			3.84			
-L·		S-622			0.06	0.07	0.01										5.72	5.68	0.04				1.01			
		+	P-6	215.00	0.00	0.00	0.00	10.52	1.79	6.86	0.46	0.00	3.16	0.01	5.91	0.19	3.75	3.50		1	-	0.116	-	8.36		
210+81.97	31.50 F	Rt. S-620			0.20	0.47	0.19					0.00					1.75	1.50	0.25	24	.00	0.102	2.66			

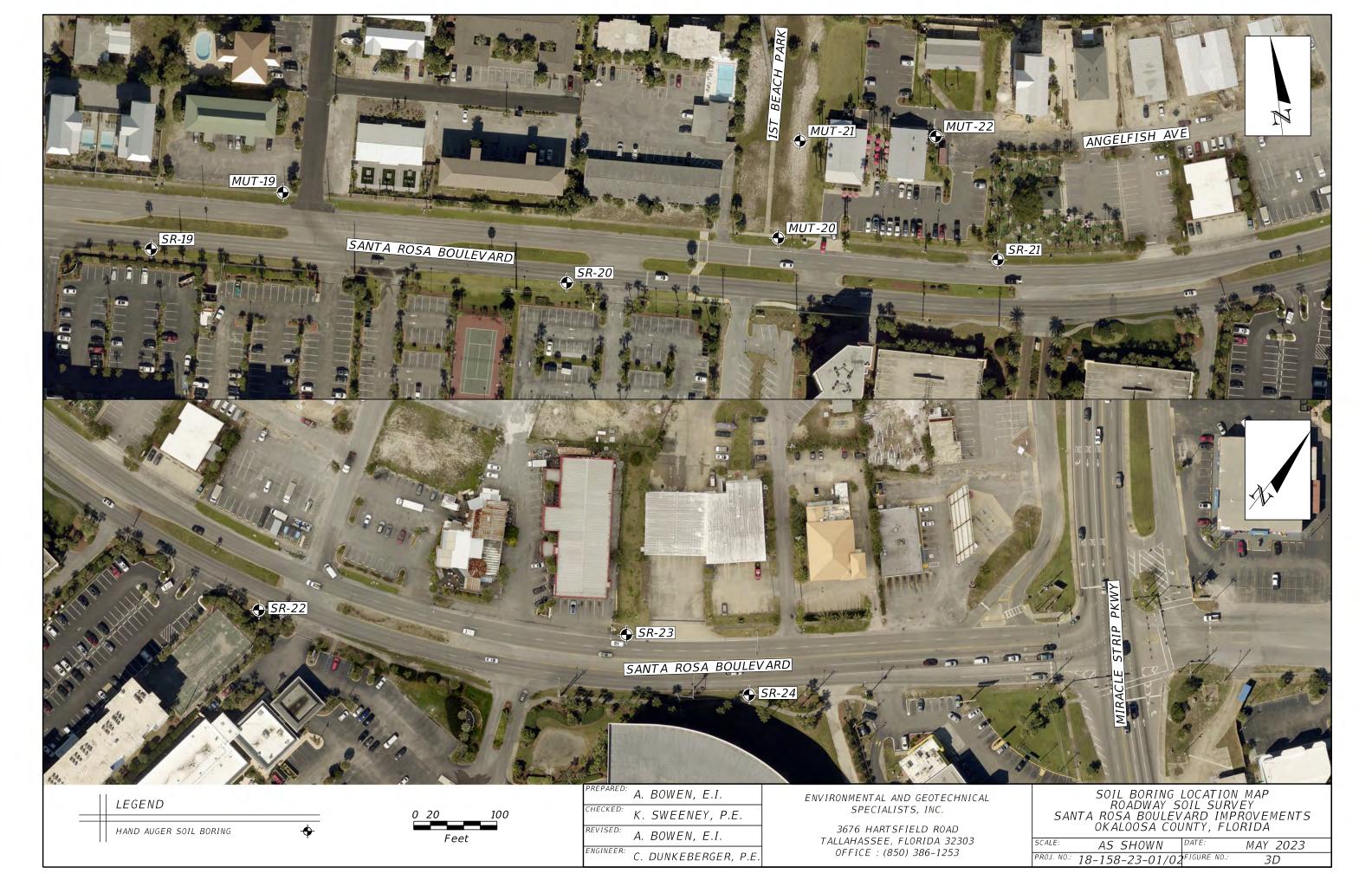
Units: ENGLISH

Page: 2











DATE OF SURVEY: MAY 2023

SURVEY MADE BY: ENVIRONMENTAL AND GEOTECHNICAL

SPECIALISTS, INC.

CRAIG DUNKELBERGER, P.E. SUBMITTED BY:

SANTA ROSA BOULEVARD IMPROVEMENTS REPORT OF TESTS

DISTRICT: 3 ROAD NO.: _--COUNTY: OKALOOSA

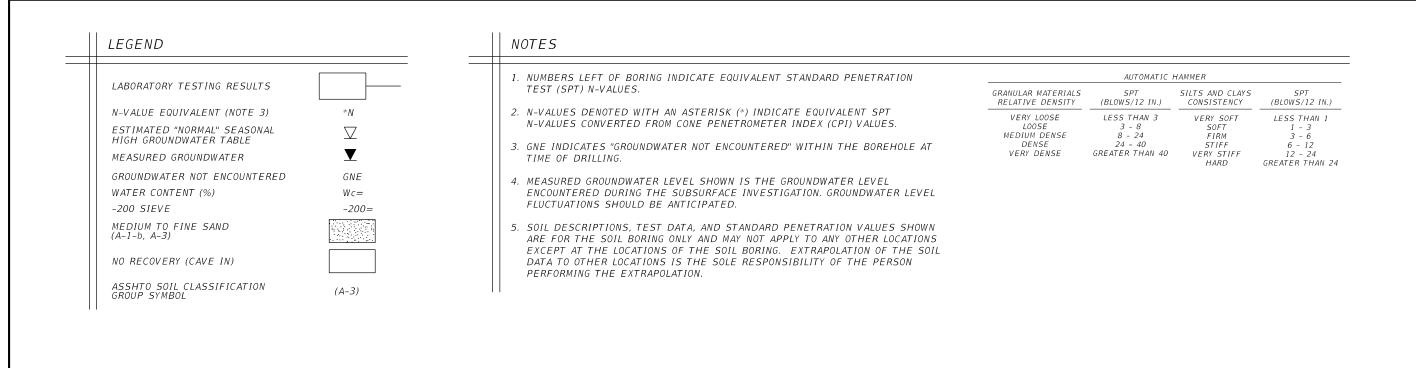
	ORGANIC CONTENT					SIEVE ANALYSIS RESULTS PERCENT PASS (%)						RG 6)			
STRATUM	NO. OF	%	MOISTURI	E_ <i>NO. OF</i>	10	40	60	100	200	NO. OF	LIQUID	PLASTIC	ASSHTO	DI	ESCRIPTION
NO.	TESTS	ORGANI	C CONTENT	TESTS	MESH	MESH	MESH	MESH	MESH	TESTS	LIMIT	INDEX	GROUP		
1				44	81-100	29-90	10-36	1-9	1-7				A-1-b, A-3	GRAY, BROWN	MEDIUM SAND TO FINE SAND

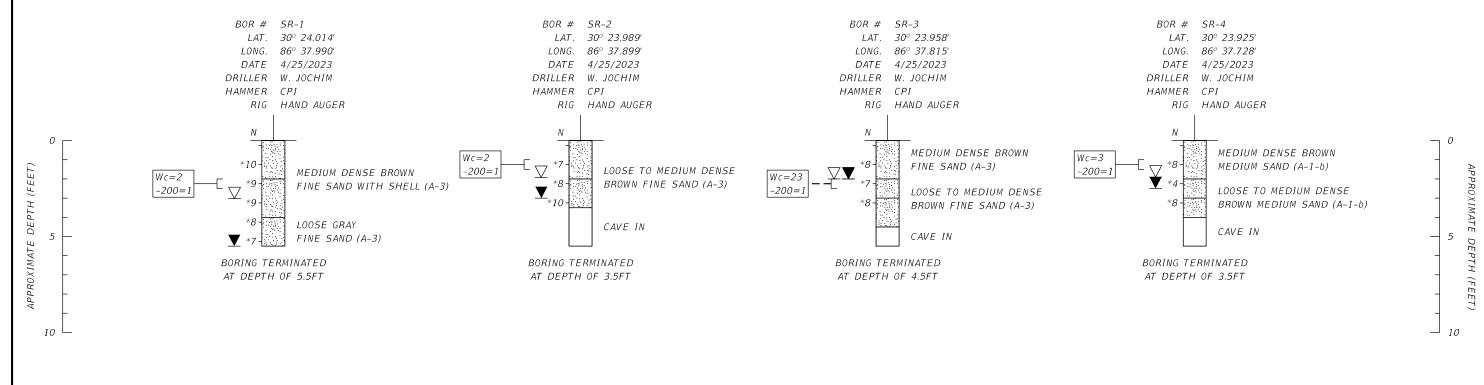
STRATA NOTES

- 1. STRATA BOUNDARIES ARE APPROXIMATE AND REPRESENT SOIL STRATA AT EACH BORING ONLY. ANY SUBSOIL CONNECTING LINES SHOWN ARE FOR ESTIMATING ONLY AND DO NOT INDICATE ACTUAL STRATUM LIMITS. SUBSURFACE VARIATIONS BETWEEN BORINGS SHOULD BE ANTICIPATED.
- 2. STRATA DESCRIPTIONS ARE BASED ON FDOT STANDARD PLANS INDEX 120-001.
- 3. STRATUM 1 REPRESENTS A "SELECT" MATERIAL.

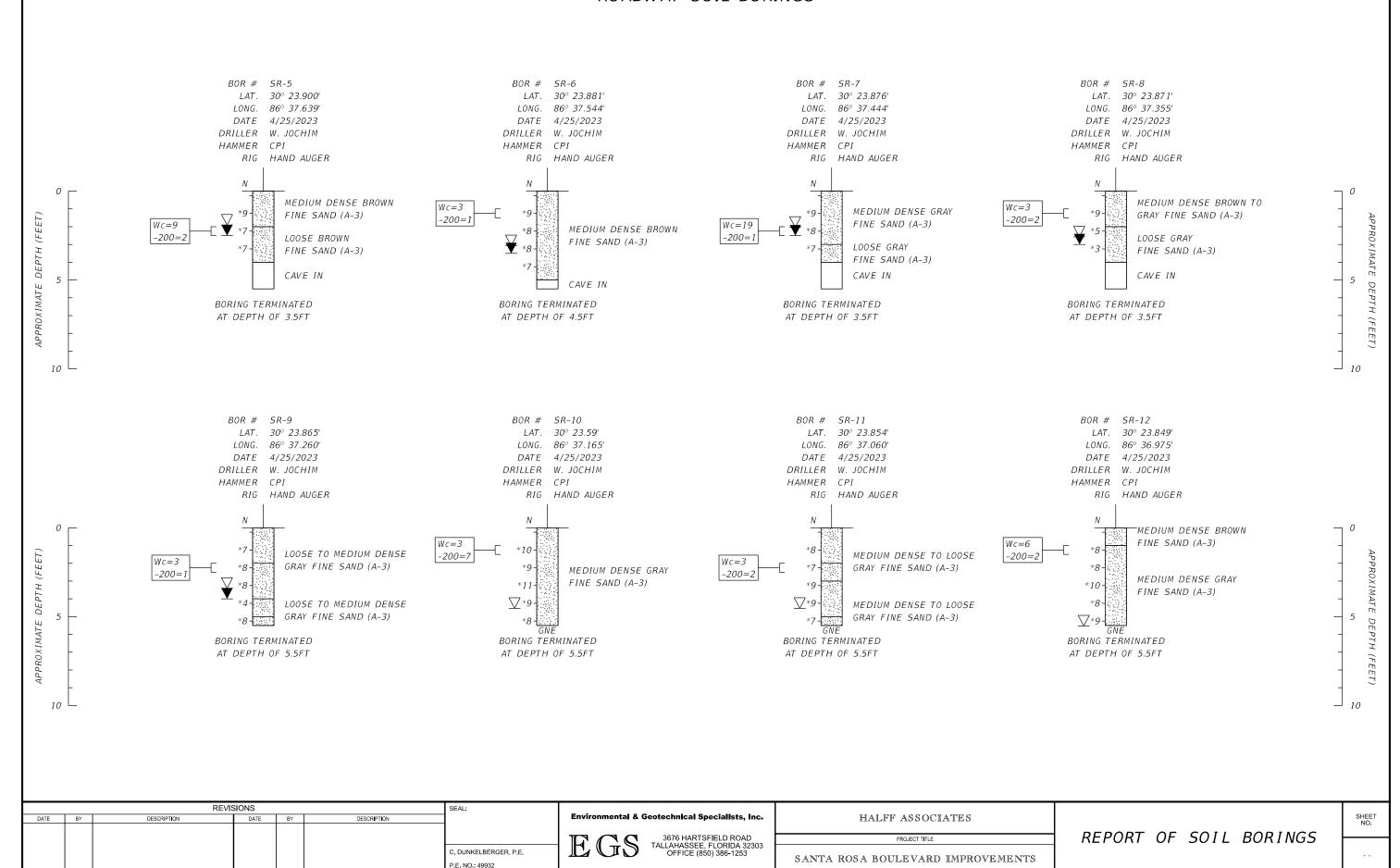
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DATE	BY	DESCRIPTION	DATE	BY	DESCRIPTION		Environmental & Geotechnical Specialists, Inc.	HALFF ASSOCIATES				
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			1				3676 HARTSFIELD ROAD	PROJECT TITLE	1			
						C. DUNKELBERGER, P.E.	TALLAHASSEE, FLORIDA 32303 OFFICE (850) 386-1253	SANTA ROSA BOULEVARD IMPROVEMENTS				
						P.E. NO.: 49932		SANTA ROSA BOULEVARD IMPROVEMENTS				

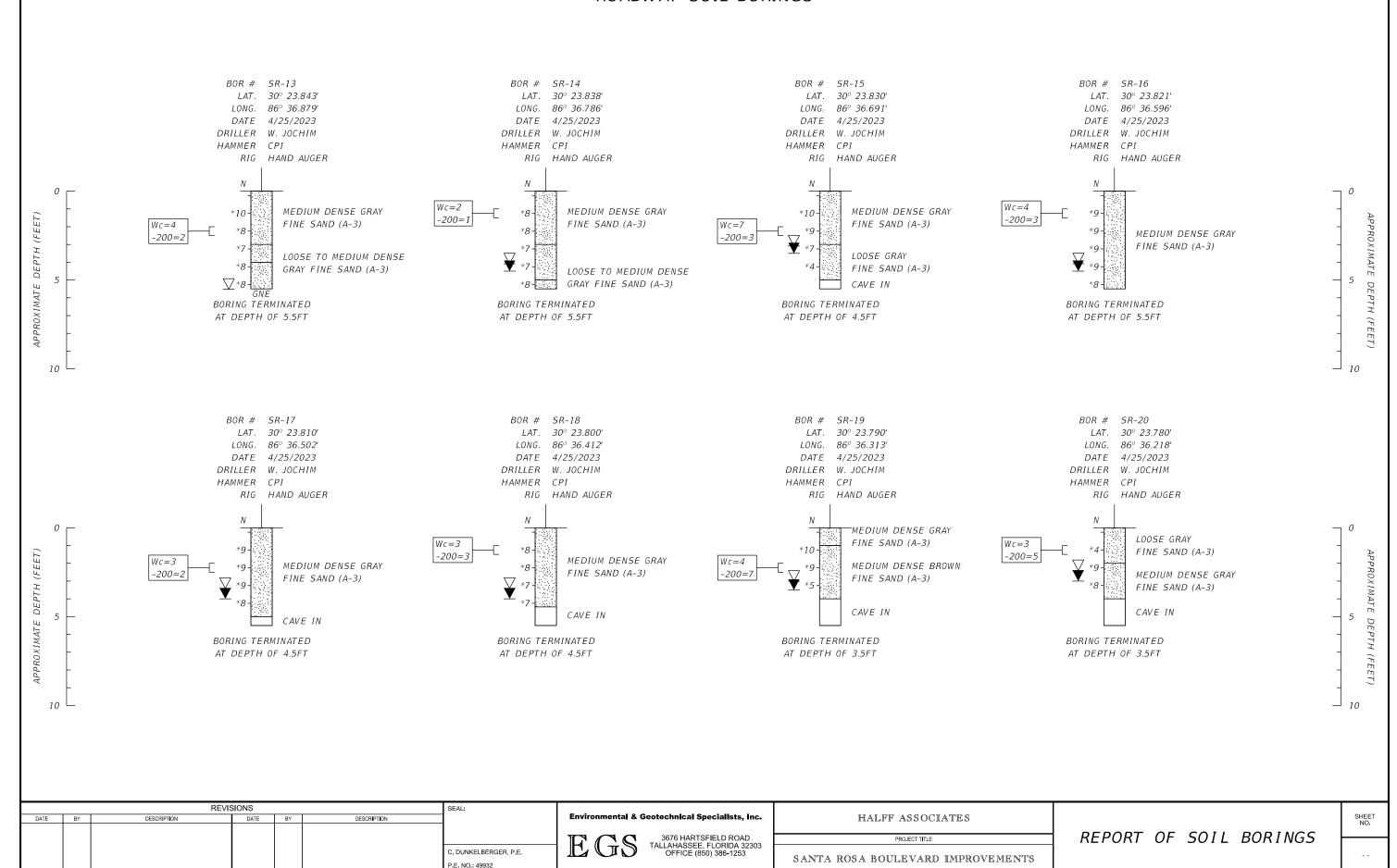
REPORT OF TESTS

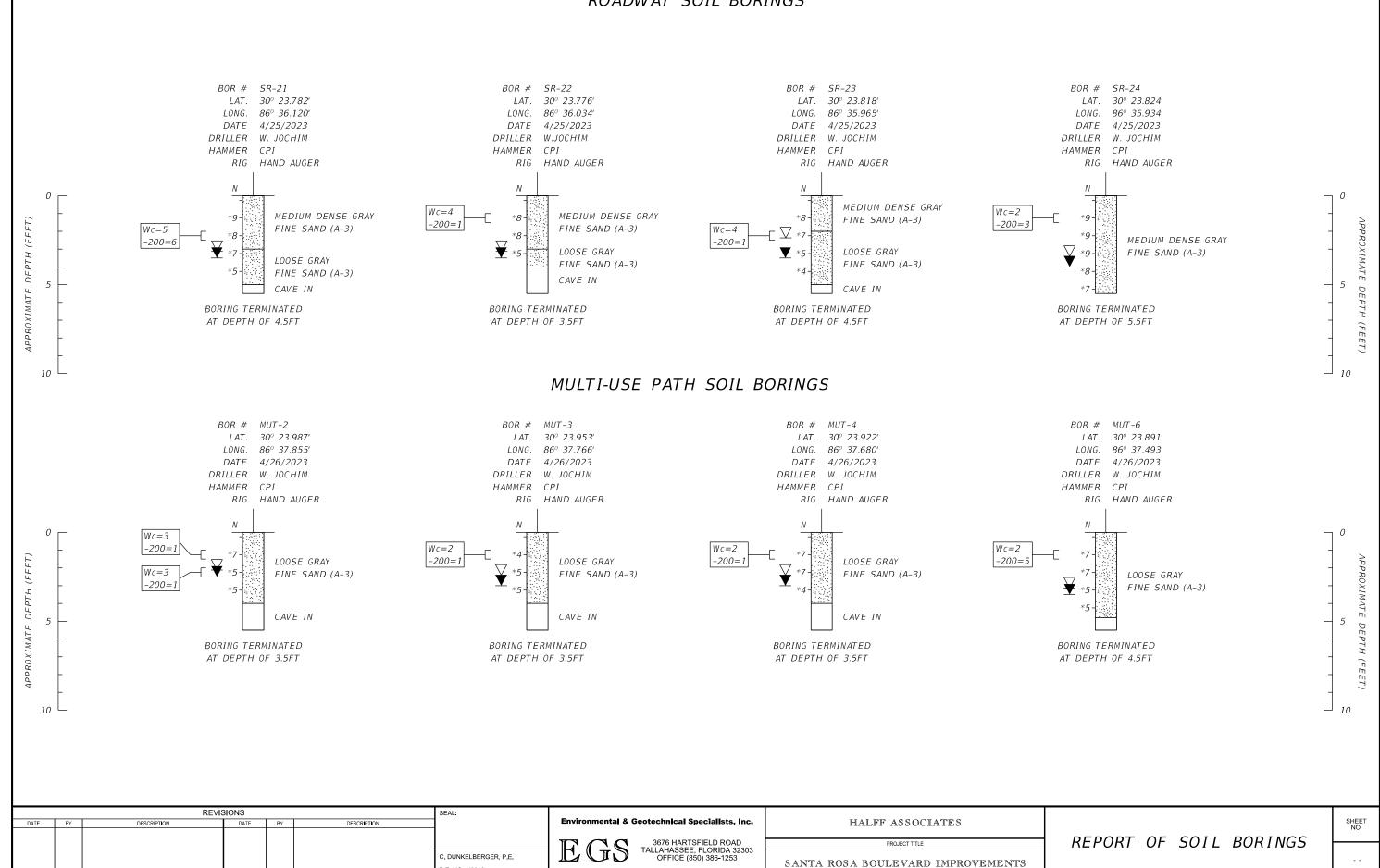






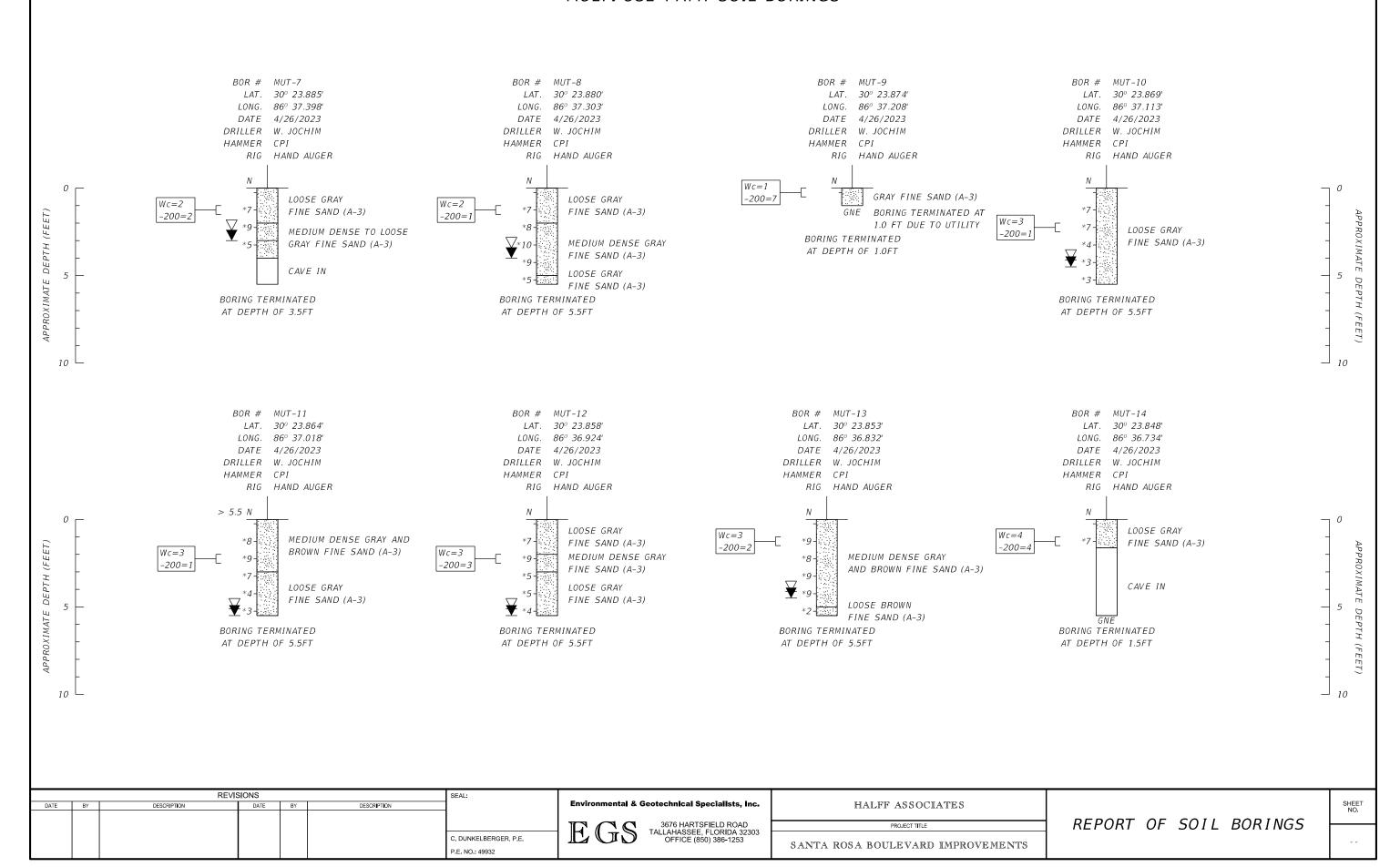




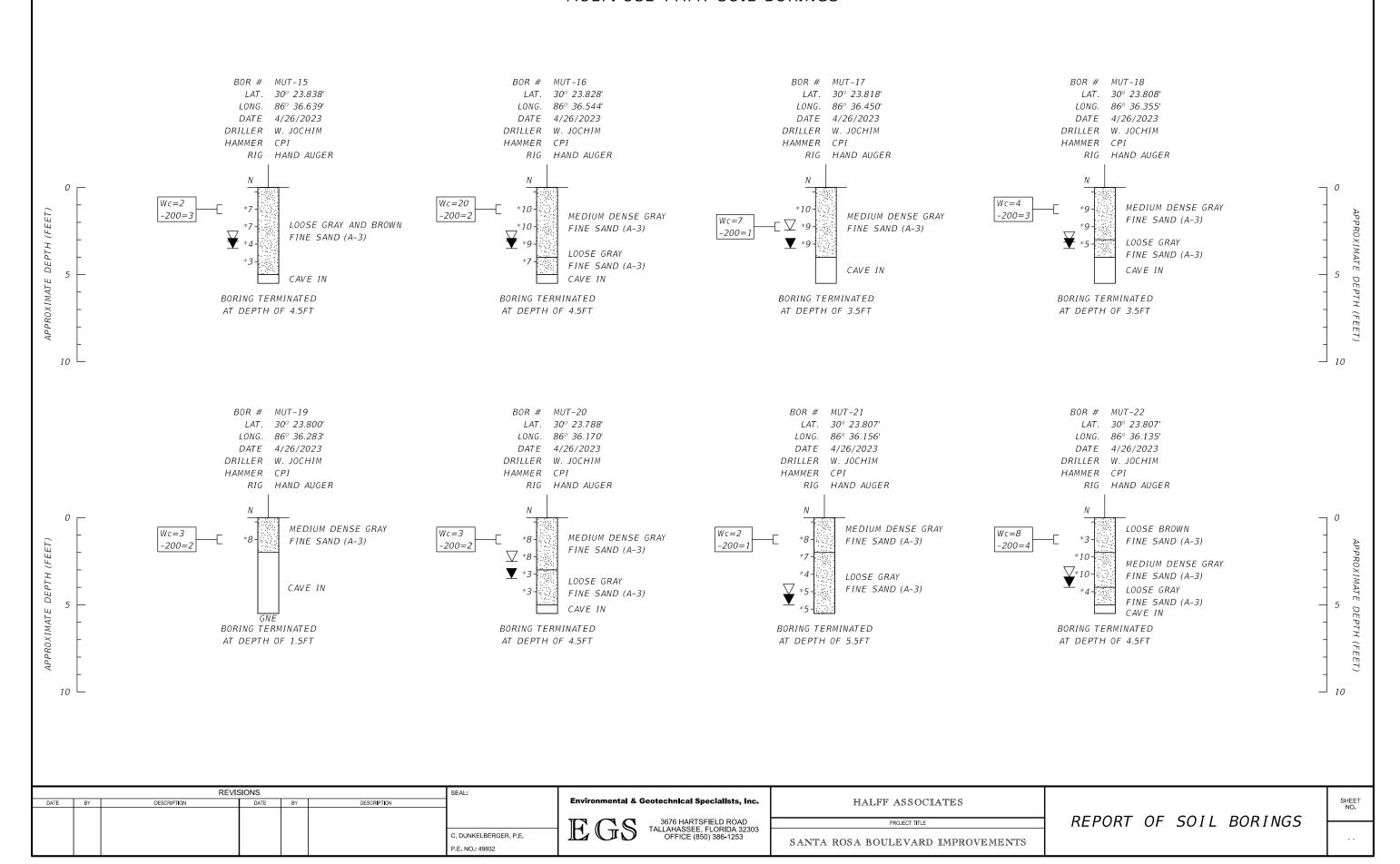


P.E. NO.: 49932

MULTI-USE PATH SOIL BORINGS



MULTI-USE PATH SOIL BORINGS





Meeting Notes:Northwest Florida Water Management District Pre-Application DiscussionProject:Santa Rosa Boulevard Roadway Improvements – Okaloosa County, Florida

Date: Wednesday, May 22, 2024
Time: 1:00 PM (Central Time)

Location: Virtual (Microsoft Teams)

Attendees: <u>NWFWMD</u> <u>Okaloosa County</u> <u>Halff</u>

Duc LeCarisse LeJeuneMurray SantoroAaron WaitsMike AlmeidaCourtney TriplettRichard Davenport

Joel Ealum

Staff Introductions and Project Discussion:

- Existing Conditions/Observations
 - Eglin AFB to East of Amberjack Drive: Four total travel (thru) lanes, two travel (thru) lanes in each direction (divided) with miscellaneous turn lanes, grassed medians, and paved crossovers
 - <u>East of Amberjack Drive to End of Project (Fire Department):</u> Four total travel (thru) lanes, two travel (thru) lanes in each direction (undivided) with paved median
 - o Concrete sidewalk along north side of Santa Rosa Blvd
 - No formal treatment areas
 - There are 7 existing outfalls located on the north side of Santa Rosa Blvd. where runoff enters before traveling over several hundred feet across sandy soils and outfalling into the Santa Rosa Sound to the north
 - Existing soils in the area consist of Type A Sandy Soils. The percolation rates obtained from testing are 52.9 in/hr.
- Proposed Roadway/Pedestrian Improvements
 - o Reduction of travel lanes for majority of project limits (reduced total impervious area by 2.37 AC)
 - <u>Eglin AFB to Park Drive</u>: One travel lane in each direction with concrete ribbon curb and roadside swales, 4' sidewalk on north side
 - Roundabout at Park Drive: Adjacent to County-owned property
 - Park Drive to 1st Beach Park: One travel lane in each direction with center two-way left turn lane, concrete ribbon curb and roadside swales, 14' Multi-Use Trail on north side
 - 1st Beach Park to East of Amberjack Drive: Two westbound travel lanes and one eastbound travel lane with center two-way left turn lane/paved median, and Type F curb & gutter, 5' sidewalk on north side, 14' Multi-Use Trail on south side
 - <u>East of Amberjack Drive to End of Project (Fire Department):</u> Match existing configuration with two travel lanes in each direction (undivided) and paved median, proposed 5' sidewalk on north side, 14' Multi-Use Trail on south side
 - Pedestrian mobility and safety improvements
 - Midblock crossings
 - 14' Multi-Use Trail
 - Significant increase in landscaping and vegetation
- Proposed Drainage/Storm Sewer Conveyance/Roadside Swales
 - o Proposed swales will have Ditch Bottom Inlets, with the grate elevation placed ~6" above the swale bottom
 - Proposed swales will provide some retention and treatment (around 1 ac-ft) for the stormwater runoff via infiltration.
 - o Infiltration rate: the maximum value of 20 in/hr will be used per the handbook
 - o No wetlands or other surface waters (OSWs) within the project corridor
 - There are 7 existing outfalls located on the north side of Santa Rosa Blvd that will be maintained in the proposed condition
 - Endangered species impacts are not anticipated
- Overall Goals of this Project
 - o Increased pedestrian mobility and safety
 - o Reduction in travel lanes and significant reduction in impervious area
 - o Formal storm sewer conveyance and treatment system
 - No impacts to existing wetlands or other surface waters (OSWs)

Mike Almeida

From: Duc Le <Duc.Le@nwfwater.com>
Sent: Thursday, June 6, 2024 1:37 PM

To: Richard Davenport

Cc: Dana Palermo; Carisse LeJeune; Mark Llewellyn, Jr.; Murray Santoro; Mike Almeida;

Aaron Waits; Courtney Triplett; Joel Ealum

Subject: Re: Pre-App: Roadway project in Okaloosa County

Attachments: 20240513_Santa Rosa Blvd_Rendering_72X36 (Full Project Limits).pdf; 20240522_Santa

Rosa Blvd Pre-Application Meeting.pdf

Good Afternoon Richard,

Thank you for your patience. We wanted to make sure we discussed this project again with our team to ensure we all come to a common understanding of what is most appropriate for this proposed development in Okaloosa County moving forward with permitting. Unfortunately, the District was not able to fit all the proposed activities under an exemption or General Permit due to specific conditions of this project.

Though we understand this development is mainly a roadway project that does contain some minor roadway safety improvements, due to the size, location, and scope of the project area, we are recommending that this development be permitted under an Individual Environmental Resource Permit (ERP). This project does appear to provide for a net reduction in impervious surfaces as well as provide a benefit to water quality and water quantity within water resources in this area. We will not be requiring the standard stormwater design criteria under an Individual ERP for this project like we normally would. This project will just need to demonstrate that this project meets the conditions of issuance in accordance with Subsection 62-330.301(1), F.A.C., in particular demonstrating that this development will not adversely affect the water quality and water quantity of the receiving waters. The following items were discussed regarding the stormwater design of this project:

- 1. Since less impervious surfaces are proposed, it is understood that less pollutants will be generated in post-development. In order to demonstrate/quantify that less pollutants will discharge offsite, a pollutant loading analysis is recommended to show that the pollutant loading in post-development is less than pre-development conditions. The District encourages the use of BMP Trains, a user-friendly software that can assist in determining the pre- and post-development pollutant loading of this development.
- 2. It appears that onsite treatment will be proposed for this development. The District typically sees treatment volume capacity for proposed systems be provided for the first 1.00 inch of rainfall over the contributing area. The geotechnical information discussed indicated sandy soils with favorable infiltration rates which could support a dry retention system as pretreatment before stormwater runoff discharges downstream. [Section 5.2, ERP A.H. Vol. II; Part 1.1.b, Section E, Form 62-330.060(1)]

- 3. Given that less impervious surfaces are proposed, this would typically demonstrate water quantity has been reduced for this site in post-development and would usually not require attenuation onsite to meet the conditions of issuance. But since the majority of the project site will still be more than 50% impervious surfaces, the attenuation of the 2-year / 24-hour storm event utilizing SCS Method Rainfall Distribution III will be required in accordance with Section 4.5.1 of ERP Applicant's Handbook Volume II.
- 4. It was determined that stormwater runoff from the development will eventually discharge into the Gulf Islands National Seashore, an Outstanding Florida Water. Upon investigation of the outfall areas along this project site, it does not appear that the existing drainage structures along Santa Rosa Boulevard will directly discharge into this waterbody. There was one except with the drainage inlet adjacent to Stewby's Seafood Shanty restaurant (235 Santa Rosa Blvd) where we were not able to be determined if the structure routes to a headwall or an outfall pipe further downstream before discharging to the adjacent waterbody. From aerials, it does not appear an outfall pipe is located out along the waterbody. The District would need clarification that all these drainage structures do not directly discharge to the OFW. If this can be demonstrated, this project would not be required to meet the design criteria under Subsection 2.0.2 of ERP Applicant's Handbook Volume II.
- 5. The application processing fee would be dependent of the project site's limits of disturbance (total area disturbed during construction). The fee schedule is as follows (\$100 discount may be applied to fee if submitting through ePermit Portal; fees below do not show this discount):

a. Less than 10 acres: \$420b. 10- less than 40 acres: \$1500

c. 40-less 100 acres: \$5000d. 100-640 acres: \$9000

6.

- 7. The typical documents to be submitted under and Individual ERP:
 - a. Section A of Application Form 62-330.060(1) with Part 4 executed by the Applicant (the entity that the permit will be issued to and who will be doing the work)
 - b. A set of construction plans (signed and sealed)
 - c. Stormwater management report with detailed project narrative and stormwater calculations (signed and sealed)
 - d. Geotechnical report (signed and sealed)
 - e. Location Map of Project site's limits of disturbance
 - f. Pre- and Post-Development Drainage Basin Map
 - g. Operation & Maintenance Plan for proposed stormwater management system(s)

If there is something that we missed in this email or if you have any additional questions/comments/concerns, please let us know and we can set up a time to discuss.